



December 16, 2020

Mr. Tony Redman
Maryland Department of Natural Resources
Environmental Review Program
Tawes State Office Building C-3
580 Taylor Avenue
Annapolis, Maryland 21401

Contract Number:
Subject: **Druid Park Lake Drive (DPLD) Complete Streets Accessibility-
Fisheries Information Request**

Dear Mr. Redman:

WSP USA, Inc., in coordination with the Baltimore City Department of Transportation (BCDOT) Planning Division, is evaluating and identifying streetscape improvements, traffic calming opportunities and multi-modal roadway enhancements for the Druid Park Lake Drive corridor from Mount Royal terrace ramp on the west side of I-83 to the Greenspring Avenue Intersection near the northwest portion of the park. The scope of this project includes existing conditions assessments including traffic and environmental resources, concept development (10% design), and community outreach. A project location map is included for your reference.

We request any information concerning resident fish and anadromous fish or additional water quality considerations within the study area. Please send your response and any comments you have on the project to Bridey Gallagher at bridey.gallagher@wsp.com. We have limited staff in the office due to the COVID-19 pandemic, but our mailing address is 1 E. Pratt Street Suite 330 Baltimore MD 21202 if you prefer to provide comments by mail. If you have questions regarding this request, please feel free to contact me at (410) 622-3614, at your convenience. Thank you for your consideration and review of the project.

Sincerely,

A handwritten signature in black ink that reads "Bridey Gallagher". The signature is written in a cursive, flowing style.

Bridey Gallagher
Environmental Planner, WSP

Enclosure
cc by email:
Pam.mcnicholas@wsp.com
korbyn.gehlbach@wsp.com

WSP USA
3rd Floor
1 East Pratt Street
Baltimore, MD 21202

Tel.: +1 410 727-5050
Fax: +1 410 727-4608
wsp.com



Larry Hogan, Governor
Boyd Rutherford, Lt. Governor
Jeannie Haddaway-Riccio, Secretary
Charles Glass, Deputy Secretary

January 26, 2021

Ms. Bridey Gallagher
WSP USA, Inc.
1 East Pratt Street
Suite 300
Baltimore, MD 21202

**RE: Environmental Review for Druid Park Lake Drive (DPLD) Complete Streets Accessibility,
Baltimore City, Maryland.**

Dear Ms. Gallagher:

The Wildlife and Heritage Service has determined that there are no official State or Federal records for listed plant or animal species within the delineated area shown on the map provided. As a result, we have no specific concerns regarding potential impacts or recommendations for protection measures at this time. Please let us know however if the limits of proposed disturbance or overall site boundaries change and we will provide you with an updated evaluation.

Thank you for allowing us the opportunity to review this project. If you should have any further questions regarding this information, please contact me at (410) 260-8573.

Sincerely,

Lori A. Byrne,
Environmental Review Coordinator
Wildlife and Heritage Service
MD Dept. of Natural Resources

ER# 2020.2051.bc

A Biological Assessment is required for construction projects (or other undertakings having similar physical impacts) that are major Federal actions significantly affecting the quality of the human environment as defined in the National Environmental Policy Act (42 U.S.C. 4332(2)(c)). For projects other than major construction activities, the Service suggests that a biological evaluation similar to a Biological Assessment be prepared to determine whether the project may affect listed or proposed species and/or designated or proposed critical habitat. Recommended contents of a Biological Assessment are described at 50 CFR 402.12.

If a Federal agency determines, based on the Biological Assessment or biological evaluation, that listed species and/or designated critical habitat may be affected by the proposed project, the agency is required to consult with the Service pursuant to 50 CFR 402. In addition, the Service recommends that candidate species, proposed species and proposed critical habitat be addressed within the consultation. More information on the regulations and procedures for section 7 consultation, including the role of permit or license applicants, can be found in the "Endangered Species Consultation Handbook" at:

<http://www.fws.gov/endangered/esa-library/pdf/TOC-GLOS.PDF>

Please be aware that bald and golden eagles are protected under the Bald and Golden Eagle Protection Act (16 U.S.C. 668 *et seq.*), and projects affecting these species may require development of an eagle conservation plan (http://www.fws.gov/windenergy/eagle_guidance.html). Additionally, wind energy projects should follow the wind energy guidelines (<http://www.fws.gov/windenergy/>) for minimizing impacts to migratory birds and bats.

Guidance for minimizing impacts to migratory birds for projects including communications towers (e.g., cellular, digital television, radio, and emergency broadcast) can be found at:

<http://www.fws.gov/migratorybirds/CurrentBirdIssues/Hazards/towers/towers.htm>;

<http://www.towerkill.com>; and

www.fws.gov/migratorybirds/CurrentBirdIssues/Hazards/towers/comtow.html.

[http://](http://www.fws.gov/migratorybirds/CurrentBirdIssues/Hazards/towers/comtow.html)

We appreciate your concern for threatened and endangered species. The Service encourages Federal agencies to include conservation of threatened and endangered species into their project planning to further the purposes of the Act. Please include the Consultation Tracking Number in the header of this letter with any request for consultation or correspondence about your project that you submit to our office.

Attachment(s):

- Official Species List
- USFWS National Wildlife Refuges and Fish Hatcheries
- Wetlands

Project Summary

Consultation Code: 05E2CB00-2021-SLI-0440

Event Code: 05E2CB00-2021-E-01073

Project Name: Druid Park Lake Drive (DPLD) Complete Streets Accessibility

Project Type: TRANSPORTATION

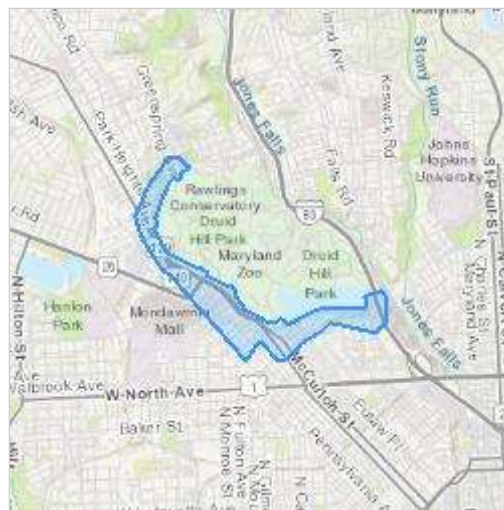
Project Description: WSP USA has been requested by the Baltimore City Department of Transportation (BCDOT) Planning Division to submit a scope of work to evaluate and identify streetscape improvements, traffic calming opportunities and multi-modal roadway enhancements for the Druid Park Lake Drive corridor from Mount Royal Terrace ramp on the west side of I-83 to the Greenspring Avenue intersection near the NW portion of the park. The required activities within the scope of this project include the completion of the following

support services:

- Project Management
- Existing Conditions Assessments
- Concept Development (10% Design)
- Community Outreach

Project Location:

Approximate location of the project can be viewed in Google Maps: <https://www.google.com/maps/@39.32194525,-76.65491684416268,14z>



Counties: Baltimore County, Maryland

USFWS National Wildlife Refuge Lands And Fish Hatcheries

Any activity proposed on lands managed by the [National Wildlife Refuge](#) system must undergo a 'Compatibility Determination' conducted by the Refuge. Please contact the individual Refuges to discuss any questions or concerns.

THERE ARE NO REFUGE LANDS OR FISH HATCHERIES WITHIN YOUR PROJECT AREA.



- Project Limits
- Jones Falls Trail
- Parks



WSP USA Inc
1 EAST PRATT STREET SUITE 300
BALTIMORE, MD 21202
(410) 727-5050

**Druid Park Lake Drive (DPLD)
Complete Streets Accessibility**

Park and Trails Map

January 2021

Cultural Resources Supplemental Memo: Select Historic Images and Brief History

MEMO

TO: Wes Mitchell, WSP, et al.

FROM: WSP Cultural Resources Group

SUBJECT: Druid Park Lake Drive and Surrounding Areas Cultural Resources

DATE: February 22, 2021

Introduction and Purpose

This memo serves to augment the initial environmental screening information provided to the project team in 2020 and early 2021. While the initial task was to focus on roadways surrounding the park, the content of this memo developed in response to questions from the team. The intent of this memo is not to provide a comprehensive park or zoo history but to highlight areas of interest that the design team may want to explore for inspiration and to also consider historic preservation compliance requirements as the project progresses and more information on the design and funding sources are confirmed.

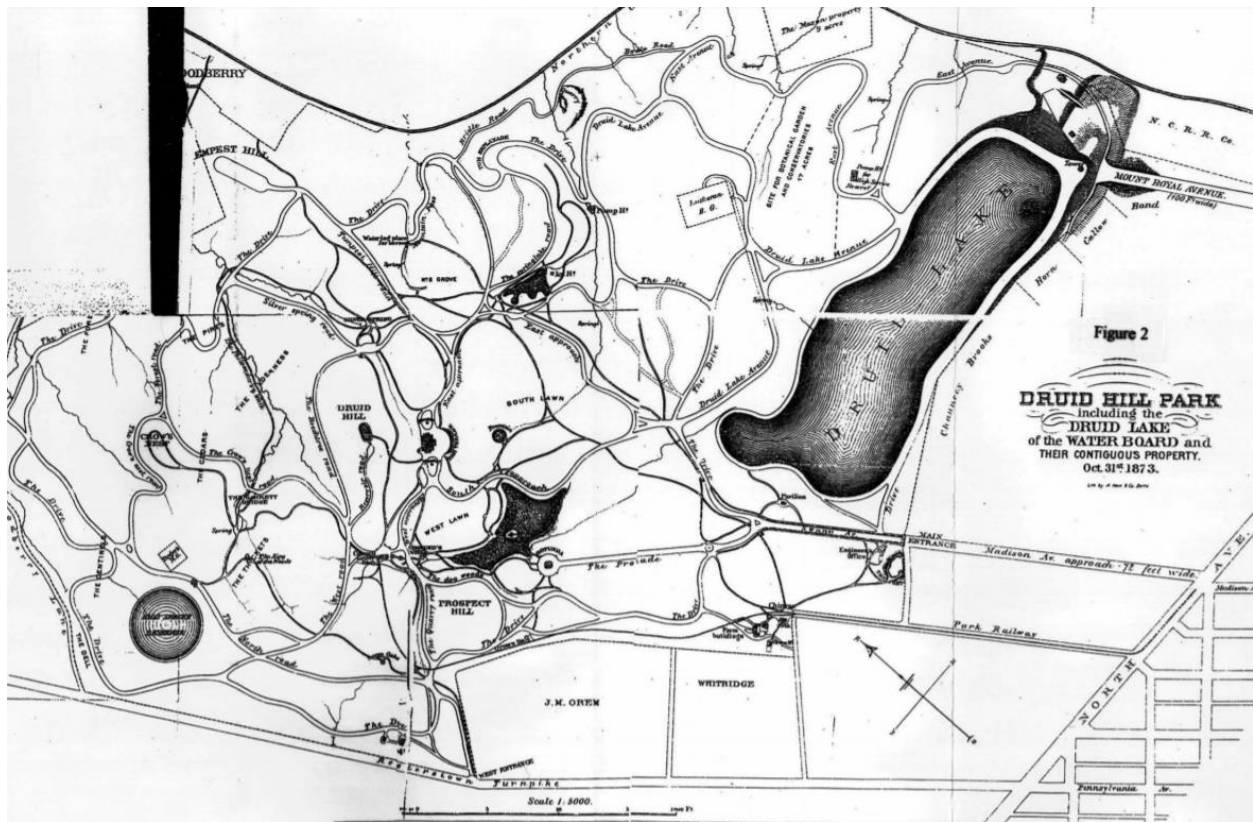
Historic Context Summary and Select Images

Early History of the Area and Establishment of Druid Hill Park

The area that now includes Druid Hill Park and the project area was initially occupied by Susquehannock Indians, who ceded land in 1652 to Lord Baltimore. The location was appealing to the Native American tribe because of its access to the Jones Falls stream and other springs in the area. Lord Baltimore subsequently began to parcel the land out.

Much of the park and project area was part of “Auchentorlie,” the estate of George Buchanan, one of the seven commissioners who founded Baltimore City in 1729. A subsequent owner, Colonel Nicholas Rogers, renamed the area “Druid Hill,” which was the name when the City of Baltimore purchased the property from his son Lloyd Rogers in 1860. Mayor Thomas Swann established Druid Hill Park later that year on October 19, 1860. A one-cent park tax on the nickel horse-car fares financed the purchase.

Druid Hill Park, which consists of 745 acres, was one of the first large public parks in America; Baltimore’s first large municipal park; and the third oldest established park in the United States. At the time that it was established, the park was on the northern most edge of Baltimore’s urban development.



1873 map of area (from the park's NRHP documentation; additional maps from later years exist)

Druid Hill Park Design

The park was designed by landscape designer and gardener Howard Daniels (1815-1863). Daniels also designed numerous park-like Victorian-era cemeteries and grounds for private residences and institutions. In the 1850s, his design for Central Park ranked fourth in the nationwide competition, losing to Frederick Law Olmsted and Calvert Vaux. Daniels toured English parks and Gardens from 1855-1856 and was influenced by the naturalistic landscapes. He opted to leave natural wooded habitats within the park, most notably in the northern areas of the park, which contains some of the oldest forest growth in Maryland.



Druid Hill Park, circa 1907

<https://www.flickr.com/photos/enochprattlibrary/albums/72157625737220141>

Daniels' designs for the curvilinear park drives contributed to the naturalistic appeal. Later, as automobiles became popular, car dealers would use these winding roads to teach new car buyers how to drive.



Druid Hill Park Reservoir, circa 1925

<https://www.mdhs.org/digitalimage/street-scene-automobiles-along-druid-hill-park-reservoir-baltimore>

Park Entrances and Exits

John H.B. Latrobe, son of renowned architect of the U.S. Capitol Benjamin Henry Latrobe, designed the colossal entrance and notable exit gateways to Druid Hill Park. The Madison Street entrance was the most prominent and monumental entrance gate. The Mt. Royal Avenue gate, which was considered an exit, featured two slightly dissimilar piers with smooth, deeply incised blocks topped with cornices and elaborate lighting fixtures, while the Fulton Street exit incorporated with spherical finials smooth piers with cornices. These piers separated more elaborate segmentally arched cast-iron gates. These gates could serve as design inspiration for new or restored park features.

Unfortunately, Daniels only lived about three years after his win to design the park. Park engineer Augustus Faul and architect George Aloysius Frederick completed Daniels' general design wishes while adding their own design vocabulary to the final result. In the early twentieth century, the celebrated Olmsted Brothers firm consulted for the city, providing advice on the park design.



Druid Hill Park, Madison Street Entrance, date unknown

<http://www.mdhs.org/digitalimage/druid-hill-park-gate-madison-avenue-entrance>



Druid Hill Park, Madison Street Entrance, date unknown

<https://www.kilduffs.com/Parks.html>

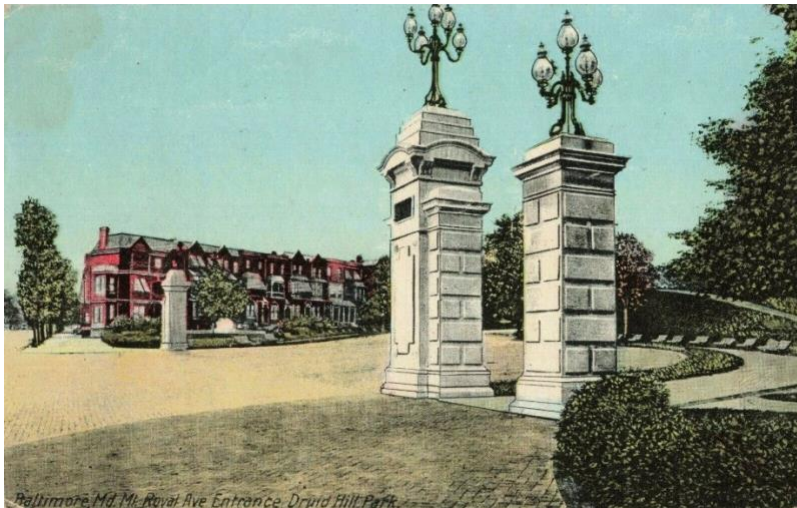


Druid Hill Park, Madison Street Entrance, date unknown

<https://www.kilduffs.com/Parks.html>



Mt. Royal Avenue from Druid Hill Park, date unknown. Note entrance piers on the right.
<https://www.kilduffs.com/Parks.html>



Close-up of Mt. Royal Ave. Entrance to Druid Hills Park
<https://www.ebay.com/itm/Postcard-Entrance-Druid-Hill-Park-Baltimore-Maryland-/352998342245>



Druid Hill Park, looking south to Madison Street entrance

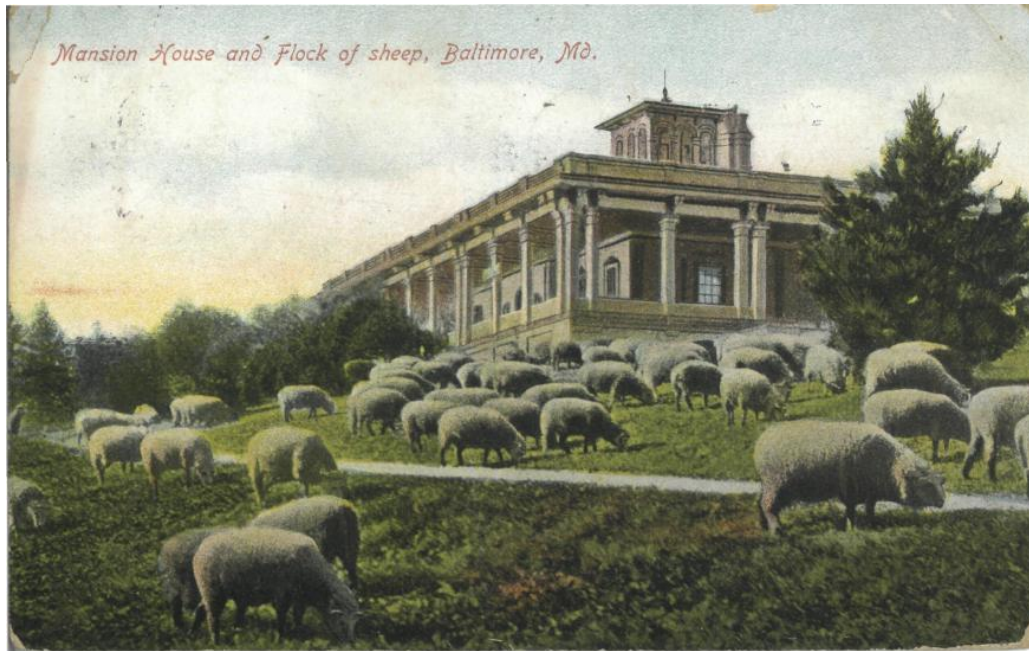
<https://www.mdhs.org/digitalimage/street-scene-druid-hill-park-baltimore>



Druid Hill Avenue at Fulton Street

<https://www.mdhs.org/digitalimage/druid-hill-park-entrance-druid-hill-avenue-fulton-street-baltimore>

At the time it was developed, the nationwide American Parks Movement was beginning to sweep the nation. The movement advocated for providing large parks for urban dwellers to allow for outdoor recreational and social opportunities, following earlier European traditions. Initially, the large grass expanses were “mowed” by a flock of Southdown sheep. The sheep were tended by a shepherd who was one of the park’s first employees.



Flock of sheep, Druid Hill Park grounds, circa 1900

<https://baltimorecityhistory.net/online-exhibit-gallery/historic-baltimore-postcards/#jp-carousel-3430>



Druid Hill Park Shepherd and Sheep, circa 1912

<https://www.flickr.com/photos/enochprattlibrary/albums/72157625737220141>

The park has several prominent features. The man-made Druid Hill Reservoir was constructed from 1863-1871 and is encircled by a popular trail for walking, running, and biking. The Maryland Zoo (previously known as the Baltimore Zoo) was established in 1876. The park also contains the Howard P. Rawlings Conservatory and Botanic Gardens (renamed in 2004), which includes the country's second oldest Victorian-era glass conservatory. Druid Hill Park also includes woodland areas, athletic fields, picnic areas, and hills for winter sledding. A portion of the Jones Falls Trail, used for hiking and biking, extends through a portion of the park. Historic monuments and statuary within the park include monuments to Scottish hero William Wallace, Christopher Columbus, and George Washington.



Historic post card of Druid Hill Park Conservatory, now the Rawlings Conservatory, circa 1930.

Other architectural follies in the park included boathouse, as well as several pavilions and pagodas with Moorish designs and numerous springs with unique designs were scattered throughout the park. Like the entrance and exit gates, these elements could serve as ongoing design inspiration.



Historic postcard image of the park's Moorish bandstand.
The structure is no longer extant. Author's collection.



This historic image shows the park's Moorish tower that remains in place. Author's collection.

Racial Segregation as Part of the Park's History

When Druid Hill Park first opened, its facilities were racially segregated. This practice continued into the twentieth century. Although championship events and competitions for various sports with African-American players were held in the park, the segregationist policies were not abolished until 1948. At that time, twenty-four Black tennis players openly challenged this discrimination by playing on the "white-only" tennis courts. Although they were arrested, their brave actions ultimately dismantled the park's segregation and influenced changes throughout the City. The names of the protestors are commemorated on the Baltimore Tennis Club marker, which is located adjacent to the Rawlings Conservatory. Interestingly, this event was the subject of renowned Baltimore writer H.L. Mencken's final *Baltimore Sun* editorial, with Mencken condemning the City's divisive policies.



African-American men playing tennis, Druid Hill Park, circa 1948
<http://www.mdhs.org/digitalimage/men-playing-tennis-druid-hill-park>



Druid Hill Park, segregated swimming pool, circa 1948
<http://www.mdhs.org/node?page=62>



An interracial group called the Young Progressives of Maryland peacefully protest the segregated tennis courts in Druid Hill Park, 1948

<https://www.theclio.com/web/entry?id=9536>

Administrative Buildings

Although over time, they have become separated from the current park boundary (apparently due to changes in circulation and traffic patterns), modest administrative parks buildings that surround the present-day park are historic and contributing to the historic district. One was built in 1894 as the Engineer's Office, later used as an administrative office, and now is a Baltimore City Parks and Recreation building; it was altered in 1955 but is still considered contributing. The other appears to have originally served as a pumping station and was built in 1873. It was later altered for use as a bath and field house in 1924; It is also contributing to the park historic district and associated with the park's significant integration history in the 1956 when Black citizens were finally allowed to use the bath house and the pool in the park.

Some of the initial DPLD concepts called for the removal of historic buildings. While moving buildings is usually strongly discouraged because it removes the buildings from their original settings, it doesn't mean that the buildings could never be moved; it just means that the Section 106 and/or historic preservation review process will likely be more difficult and longer, and the team may not get the desired outcome or we may present risk to the client by spending project funds while potentially not being able to execute the project as planned. A design that reintegrated these buildings within the park may be more successful.

Additional Ideas for Consideration

A cursory list of ideas to be explored may include ways for the park's original entrances to be safely reintegrated into the pedestrian experiences in the park, particularly for local residents who find safe access to be difficult despite living proximate to the park. Reintegrating these entrances, particularly the monumental Madison Street gate and the exits at Mount Royal Avenue and Fulton Street, perhaps in tandem with landscaping and paving materials to remind drivers that they are traversing a pedestrian area, could restore both historic setting and safety.

Another consideration could be reintegrating some of the unique fountain designs within the park or using the spring names to delineate historic segments of the park or in wayfinding signage. Numerous fountains existed in the park, many with distinguishing forms or sculptural features. Most were closed when surrounding development caused water contamination, depriving park users of a welcome source of refreshment during the hot summer months.



A stock photo of the Crises Fountain in Druid Hill Park, collection of New York Public Library.

Opportunities for highlighting Black history are also present and should not be overlooked. The story of integrating the park coupled with the desire to remove and/or rededicate statues of George Washington, Christopher Columbus, et al. open opportunities within the park to highlight other aspects of park history or African-American leaders. Transportation-related history of the park could also be interpreted and buildings such as the Mansion House, Conservatory, and unique zoo structures could also prove to be a draw. Community input is particularly important for these ideas to determine what park users and neighbors.

Current Designations and Historic Preservation Compliance Considerations

Built Historic Properties

Although an Area of Potential Effects (APE) has not been delineated, a general Preliminary Study Area has been developed. This area includes built historic properties, which are those that are listed in or eligible for the National Register of Historic Places (NRHP). Only historic properties are subject to compliance with federal historic preservation law and they are assessed for effects from the project's physical impacts as well as visual, auditory, atmospheric, or vibratory effects from the project. The APE will be established when the project's limits of disturbance are confirmed.

Many prominent built historic properties, including both historic districts and individual resources, are within the Preliminary Study Area. Some, such as the Reservoir Hill Historic District (B-1379) are listed in or determined eligible for the NRHP, while others have only been identified but not evaluated for NRHP eligibility. Properties more than fifty years of age within the APE are generally evaluated for NRHP eligibility. Some of these properties may not have been identified or evaluated previously.

NRHP-listed or eligible properties, including both built resources and archaeological sites, are subject to compliance with Section 106 of the National Historic Preservation Act and its enabling legislation found at 36 CFR Section 800. This is required if the project will receive federal funding or permitting. Compliance with Baltimore City's Commission of Historical and Architectural Preservation's (CHAP) regulations is required for Baltimore City's designated local historic districts and landmarks. As more project information is confirmed, qualified cultural resources staff will coordinate with city project management staff, CHAP, and State Historic Preservation Office staff, as appropriate, to make sure the project complies with applicable local, state, and federal historic preservation laws.

Although numerous historic properties are present within the Preliminary Study Area, the most prominent and relevant is Druid Hill Park (B-56), which was listed in the National Register of Historic Places in 1973. A substantial update of the original registration form was completed in 1997. This updated documentation identifies 22 contributing buildings; 4 contributing sites; 24 contributing structures; and 20 contributing objects.

Archaeological Sites and Related Considerations

Information on archaeological sites and survey efforts were obtained by a professional archaeologist meeting to Secretary of the Interior's Professional Qualifications Standards from on-line Maryland Historical Trust (MHT) records (accessed November 2020 – January 2021). Additional online digital data sources were used in the assessment of archaeological potential, including historic mapping, aerial photography, and soils and geology data as well as LiDAR imaging.

Based on current MHT records, there is only one previously identified archaeological site located within the current Preliminary Study Area for the Druid Park Lake Drive project. The Druid Hill Park Superintendent's House archaeological site (18BC176) is situated in the triangular parcel bound by Auchentoroly Terrace, Liberty Heights and Reisterstown Road. This site does not appear to have had a formal Determination of Eligibility (DOE) for the National Register of Historic Places (NRHP). However, given the documented nineteenth-century structural remains and archaeological deposits, as well as the potential remains from an earlier eighteenth-century plantation, the site should be considered potentially significant and will need to be formally evaluated. Site 18BC176 falls completely within the currently delineated Preliminary Study Area, and if eligibility is confirmed, planning would need to consider attempts to avoid, minimize or mitigate potential adverse effects. There is one additional archaeological resource recorded in the general vicinity of the current Preliminary Study Area: 18BC100, which is the remains of an earlier eighteenth-nineteenth century industrial mill dam, located east to the Johns Falls Expressway, 403 feet (122 meters) from the nearest point of the current Preliminary Study Area.

MHT records do not show any other previously completed archaeological survey efforts in the vicinity of the current project. Druid Hill Park (specifically the Maryland Zoo area) is reported to contain at least two cemeteries (one of which has African American associations) and these have been tentatively located based on historic mapping. The smaller cemetery is in the north-western portion of the park, 781 feet

(233 meters) from the closest point of the currently defined Preliminary Study Area. The larger cemetery is shown in the south-central part of the park, separated from the Preliminary Study Area by Druid Hill and approximately 1,657 feet (510 meters). Potential project effects to either cemetery or Site 18BC100 are not anticipated.

Archaeological Potential

In addition to effects to recorded archaeological sites, ongoing cultural resource studies will consider potential effects on previously unidentified archaeological resources. Within the Preliminary Study Area, areas of park property could be considered to have generally higher potential to preserve the integrity of historic landscapes and any archaeological sites they might contain. However, given the intensity of the landform modification required to create the park, pre-contact or early historic site preservation is likely to be low. The potential for historic archaeological sites and features associated with the 150+ year-old park and zoo also should be taken into consideration. For the portions of the Preliminary Study Area outside the park, mostly characterized by relatively dense rowhouse residential and light commercial development, an assessment of potential of urban archaeological resources will need to be conducted. In general terms, existing roadways and sidewalks of such urban neighborhoods have a relatively low potential for archaeological resources, compared to landscaped areas, backyards, open lots and alleys. There also are several churches within the Preliminary Study Area and the location of associated graveyards and the potential for unmarked burials may also need to be assessed.

Additional Research Potential

In addition to the initial environmental screening for built historic properties and archaeological sites presented to the project team, numerous Maryland Inventory of Historic Properties forms and survey forms for surrounding buildings as well as for the park itself provide information. These include but are not limited to those inventoried by the Maryland Historical Trust; CHAP; and the National Register of Historic Places. When an APE is developed and both funding and permitting sources are confirmed, those within the Area of Potential Effects will need to be assessed for historic significance if they have not previously been subject to evaluations and those that are significant for historic or architectural reasons, i.e., meeting established federal, state, and/or local criteria, will be assessed for effects from the project.

Additional Details and Future Research

Finally, the well-researched, erudite, delightfully written book *Druid Hill Park: The Heart of Historic Baltimore* by Eden Unger Bowditch and Ann Draddy provides images and text useful to understanding the park's history and importance of place within Baltimore and the entire state. It is a reliable source to add accurate details to the cursory information provided here.

Appendix D

Design Considerations

- Druid Park Lake Drive Analysis Segments
- Landscape and Urban Design Analysis

Druid Park Lake Drive Analysis Segments

The Project Team performed an initial analysis of issues along the corridor by segment. These segments were selected based on similar land use and roadway characteristics and allowed for detailed examination in the initial concept development process. The issues identified in these segments informed opportunities for design elements.

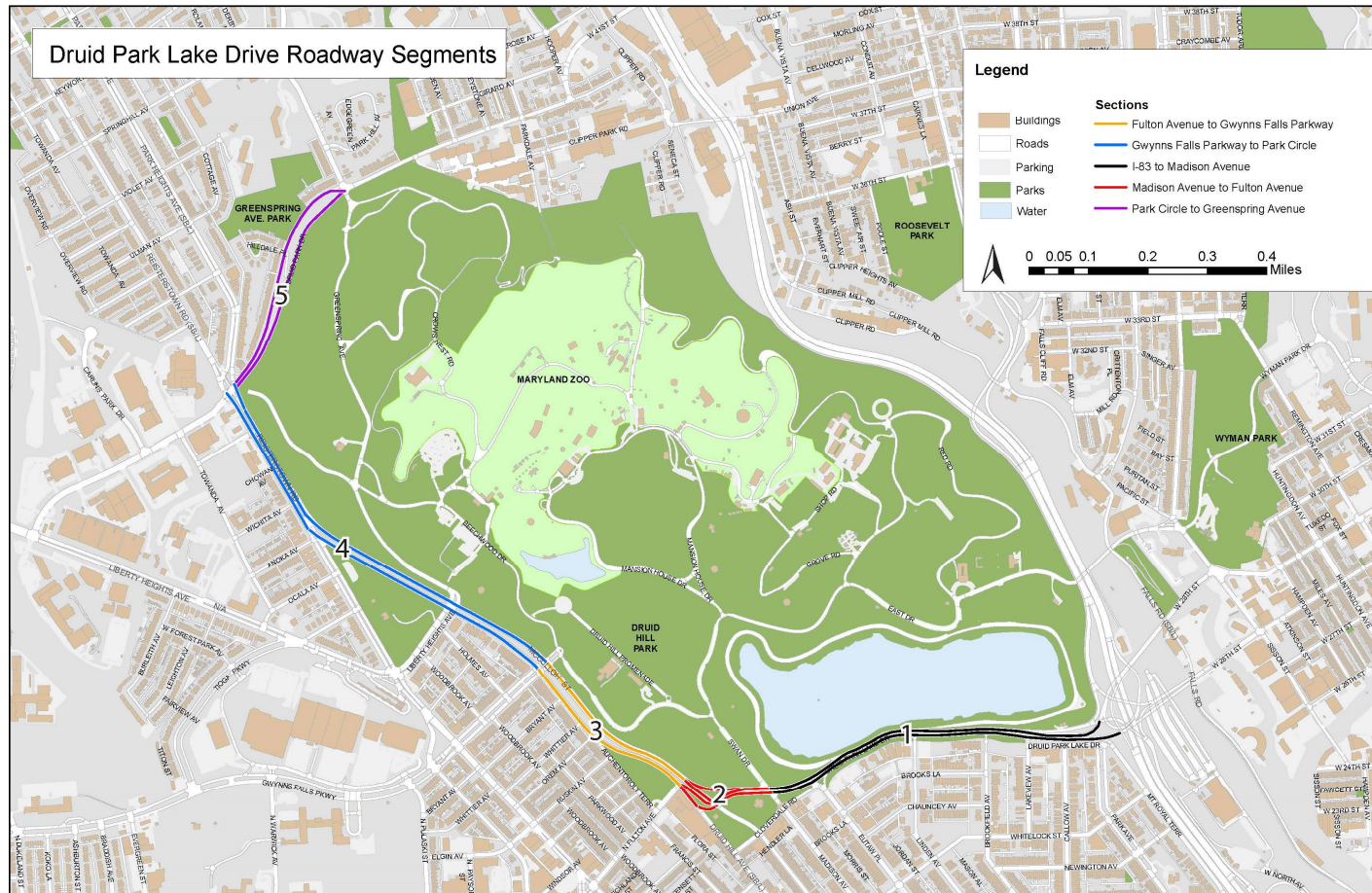


Table 1: Druid Park Lake Drive Existing Conditions Analysis - Roadway Segments

| Druid Park Lake Drive Roadway Segments | | | | | | |
|--|-------------------------------------|-------------------|-----------------------|------------------|------------------|---|
| Segment Number (See map above) | Segment Boundaries | Segment Length | # of Intersections | # of EB Lanes | # of WB Lanes | Characteristics |
| 1 | I-83 to Madison Ave. | 0.60 mi | 7 | 2-3 | 1-2 | The Big Jump temporary shared-use path is on the EB side of this segment. Ongoing construction on this segment related to Druid Lake water tank project. Large apartment buildings and 1-2 development sites to consider. Major entrance to Druid Hill Park at Madison Ave. Potential opportunity for new park entrance(s). |
| 2 | Madison Ave. to Fulton Ave. | 0.15 mi | 3 | 3-5 | 3-5 | Complex and unsafe intersection. Transition from McCulloh St. to DPLD and from DPLD to Druid Hill Ave. Large green spaces in median and adjacent to roadway. |
| 3 | Fulton Ave. to Gwynns Falls Parkway | 0.3 mi | 5 | 3 | 3-5 | Existing local access lane (Auchentoroly Terrace). Minor intersections with residential roads on EB side. Potential opportunity for new park entrance(s) and local access lane. Landscaped median. Major entrance to Druid Hill Park at Gwynns Falls. |

| | | | | | | |
|---|-------------------------------------|---------|---|-----|-----|--|
| 4 | Gwynns Falls Parkway to Park Circle | 0.72 mi | 6 | 2-3 | 2-4 | Existing cycle track. Transition to Reisterstown Rd. Minor intersections with residential roads on EB side. Newly reconstructed intersection at Park Circle. Major entrance to Druid Hill Park at Greenspring. |
| 5 | Park Circle to Greenspring Ave. | 0.38 mi | 2 | 1-2 | 1-2 | Residential scale. Limited ROW. Large green median with relatively steep slope. |

DRUID PARK LAKE DRIVE COMPLETE STREET DESIGN

Landscape/Streetscape/Urban Design Analysis - Abbreviated Narrative

INTRODUCTION

DPLD possesses intrinsic qualities that make it a unique street unlike others. On one side, DPLD surrounds half of the Druid Hill Park's border, one of the most prestigious city parks in the Country, designed by the father of landscape architecture, Mr. Frederick Olmsted. On the other side of DPLD are historic landmarks, a city and national registered historic district, buildings with architectural significance, vacant lots, parklands, tree groves, hedge rows, low masonry walls, and vegetative slopes. For about half of the DPLD, there are tree planted medians in the middle of the street, which provide the boulevard feel and is a significant asset to preserve.

KEY ISSUES - Landscape/Streetscape/Urban Design

- Community's accesses and connections to the Park.
- Need to accommodate all users of DPLD safely and efficiently: motorists (commuters and residents), pedestrians (all ages and physical conditions, park visitors and residents), and bicyclists (park visitors and residents).
- The speed of traffic is too fast for the motorist to experience and appreciate the historic, architectural, landscape and ecological heritage of the corridor.
- Not well-maintained street trees and other vegetation displaying an unmanaged appearance
- Need to have a more cohesive image and a stronger identity.
- Change the perception of DPLD. – from single function roadway to a multi-modal complete street that also connects the neighborhood residents to the Park of national and historic significance, so that DPLD is no longer perceived a barrier between the Park and the neighborhood.

GOALS – Landscape/Streetscape/Urban Design, Preliminary

(to be revised with the input and coordination with the project team and the community)

1. Enhance the “park” feel along the corridor.
(Prefer installation of tree groves to evenly spaced street trees.)
2. Provide a more relaxed, enjoyable and leisure driving experience.
(through various improvements.)
3. Provide a permanent, continuous hiker/biker pathway along the corridor and multiple safe, easy, and pleasant connections from the neighborhood to the park.
(a complete street environment.)
4. Reduce the sense of scale along vehicular travel lanes.
(Narrow street width to increase and consolidate bicyclist and pedestrian space. See #8.)
5. Explore the possibilities of the separation of vehicular space and pedestrian/bicyclist space, which will increase the space to provide pedestrian/bicyclist amenities.
(See picture below.)



Use Planting Median to Separate Vehicular Traffic and Pedestrian and Bicyclist Traffic and Provide Pedestrian and Bicyclist Amenities.

6. Provide the user with the opportunities to experience, explore and appreciate the historic, architectural, landscape and ecological heritage of the corridor and the area. In another word, reconnect communities to the Park's history, programs and activities and ecology.
(Reduce speed limit and calm the traffic via various means.)
7. Establish a cohesive (not unified) image and a stronger sense of place/identity.
(Use existing and proposed features and selectively repeat them along the corridor: stone walls, tree groves, hedge rows, same style of streetlights and traffic signals, paving or street paint at intersections, etc.)
8. Maintain/enhance the existing characteristics of DPLD: view to the lake, scattered tree groves, and boulevard feel of the western portion of DPLD.

STUDY SEGEMENTS

The entire project area is divided into six segments based on existing features on both sides of DPLD. Prototypical design strategies can then be developed for each segment.

- a. **I 83 to Eutaw**
South: mixed uses
North: Park
- b. **Eutaw to Ruskin**
both sides: Parkland
- c. **Ruskin to Liberty Height-**
South: Residential
North: Park
Middle: Planting Medians
- d. **Liberty to Reisterstown**
both sides: Parkland
Middle: Planting Medians
- e. **Reisterstown to Park Circle**
Northeast: Park
Southwest: Mixed use
Middle: Concrete Median
- f. **Park Circle to Greenspring**

East: Park

West - Residential

Existing Edges of DPLD –

| | |
|------------|--|
| Hard Edge- | building, walls, fence, curb |
| Soft Edge- | tree grove, hedge rows, landscape planting, wilderness - woods |

PURPOSES OF THE ANALYSIS

1. To get a sense of how the street space of DPLD is defined and how these defining elements/features or lack of these elements/features affect the user's experience, positive or negative.
2. To seek common features of each segment listed above that can be enhanced or improved.
3. To determine what landscape/street design ideas would be most appropriate for each segment.
4. To identify locations that place-making strategies can be implemented to strengthen the cohesiveness along the corridor and the identity of DPLD.

Visual analysis is a 2D graphic interpretation of the current user experience to be used as a basis for developing design concepts that will improve and enhance the user experience. **Corridor analysis** identifies the design issues and opportunities along the corridor and what can be done spatially to achieve project goals, that is to create a complete street environment. **Intersection Analysis** identifies the issues of connecting to the Park and the place-making strategies to increase the sense of place and to improve the pedestrian friendliness as well as universal accessibility.

PRELIMINARY OVERALL CORRIDOR DESIGN VISION

- Relocate curb toward the center of the street to define the street and consolidate the space for one or two existing travel lanes and existing sidewalk spaces into one much wider pedestrian/cyclist space. This will reduce the pedestrian crossing distance from the neighborhood to the Park as well as the perceived scale of the vehicular space. See the example below for reducing the excessively wide median width and travel lane width to make room for hiker-biker trails on both sides of the roadway within the existing right-of-way.



Narrowed Median and Travel Lanes

- Narrowed street space along with smaller turning radius at all street corners can calm the traffic.
- Consolidate space with existing sidewalk to provide the pedestrian/bicyclist amenities. Provide planting, possibly site furniture and wayfinding signage. See pictures below.



- Apply place-making strategies for all major pedestrian crossing intersections to provide sense of place at intersection and cohesiveness along the corridor. The common and repeating features such as ornamental traffic signal device and street light combination, pavers or street print at pedestrian crossing intersection will contribute to the cohesiveness of DPLD. (See Summary of Intersection Analysis.)
- Place **tree groves of a mixture of major and ornamental deciduous trees** in medians and roadsides (not evenly spaced street trees). New scattered tree groves are better fit to and enhance the image of DPLD as a “park drive”, as there are many tree groves on both sides of the street.
- Explore the possibilities of introducing traffic circle(s) or roundabout at selected intersection(s) such as Fulton, Gwynns Falls, or Liberty Heights and eventually at I-83 to calm the traffic and simplify the traffic pattern.
- When necessary, due to the cut into existing slopes, use stone or cultured stones as the wall materials to be consistent with existing walls.
- Eliminate medians in Segment 1.
- Include and/or repeat some of the existing and future site features as part of the design vocabulary along the corridor, such as stone walls, tree groves, street paving, traffic signal pole and light pole, site furniture, etc. to achieve cohesiveness of the entire corridor streetscape.
- Include wayfinding signage at strategic locations will further enhance the user experience (pleasantness, ease, and welcoming) along DPLD corridor. Sign design should relate to current Park sign in colors, materials, and typeface. Minimize cluttering of various signage.

SUMMARY OF INTERSECTIONS ANALYSIS

In addition to the analysis of the six segments of the entire corridor, site analysis of selected seven intersections was also performed. These include the intersections of DPLD with Linden Avenue, Eutaw Place, Madison Avenue, Fulton Ave, Gwynns Falls Parkway, Liberty Heights, and Greenspring Avenue. The newly constructed Intersection at Park Circle is not included.

Goals

- Make each intersection a pedestrian “place”.
- Enhancing/Establish safe and welcoming pedestrian connections to the Park.
- Ensure universal accessibilities.
- Improve the visual quality of each intersection, which has similar design character.

- Each imageable intersection will contribute to the cohesiveness of the entire corridor.

Preliminary Overall Intersection Improvement Approaches

- Use place-making strategies at each intersection to enhance the sense of place at each intersection.
- Use the enhanced pedestrian crossing as a traffic calming measure for the resident, the motorist, and the hiker/biker.
- Instead of striping or installing decorative paving only on crosswalks, special street surface treatments at the entire intersections, such as StreetBond, pavers or imprinted concrete, can be installed to calm the traffic, enhance sense of place for all users, and add rhythmic experience to the motorist. (See the example below.) Simple pedestrian-scale artwork that relates to the identity or the history of adjacent neighborhoods, can also be incorporated into the street surfacing design.



- ADA compliant sidewalk and street corners – Rebuild ped ramps, 2 at each corner. Widen adjacent sidewalk to be 5' wide min. Expand paving at street corners for pedestrian stopping and congregation.
- Landscape Enhancement – Plant street trees along sidewalks in the vicinity of the intersection.
- The median – Use it as a pedestrian refuse. Place wayfinding sign.
- Traffic Signals – Install period style traffic signal poles with mast arms and streetlights combination to minimizing the cluttering of various poles (See picture below.). Avoid poles and pedestrian flow conflicts as well.



Period Style



Contemporary Style

- Incorporate existing features nearby: wall, gateway, sign, pillar, etc., into the streetscape.

- Improve connections with nearby sidewalks/trails.
- Preserve the view to the Lake, important focal point, significant building, historic features, landmark, and other visible site features.
- Reduce the distance between curbs to minimize the pedestrian crossing distance. No bump-out is proposed on DPLD.

Historic DPLD Crossings

In addition to the above-mentioned intersections, there are 16 additional locations that were identified as “historic connections” to the Park by the community. (See the image below.) It will be up to the community and the project team to jointly explore the feasibility and decide which would be revived.



Appendix E

Traffic Analysis

- Traffic Synchro Analysis Results

DPLD Existing Traffic Volumes (AM)

HCM Signalized Intersection Capacity Analysis

1: Linden Ave & Druid Lake Park Dr

12/20/2021

| | → | ↘ | ↙ | ← | ↖ | ↗ |
|-----------------------------------|-------|-------|------|---------------------------|------|------|
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | ↰ | | ↱ | ↰↱ | | |
| Traffic Volume (vph) | 691 | 15 | 0 | 954 | 0 | 0 |
| Future Volume (vph) | 691 | 15 | 0 | 954 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.0 | | | 4.0 | | |
| Lane Util. Factor | 1.00 | | | 0.95 | | |
| Frt | 1.00 | | | 1.00 | | |
| Flt Protected | 1.00 | | | 1.00 | | |
| Satd. Flow (prot) | 1857 | | | 3539 | | |
| Flt Permitted | 1.00 | | | 1.00 | | |
| Satd. Flow (perm) | 1857 | | | 3539 | | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 751 | 16 | 0 | 1037 | 0 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 767 | 0 | 0 | 1037 | 0 | 0 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 6 | | 5 | 2 | | |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 110.0 | | | 110.0 | | |
| Effective Green, g (s) | 110.0 | | | 110.0 | | |
| Actuated g/C Ratio | 1.00 | | | 1.00 | | |
| Clearance Time (s) | 5.0 | | | 4.0 | | |
| Vehicle Extension (s) | 3.0 | | | 3.0 | | |
| Lane Grp Cap (vph) | 1857 | | | 3539 | | |
| v/s Ratio Prot | c0.41 | | | 0.29 | | |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 0.41 | | | 0.29 | | |
| Uniform Delay, d1 | 0.0 | | | 0.0 | | |
| Progression Factor | 1.00 | | | 1.00 | | |
| Incremental Delay, d2 | 0.7 | | | 0.2 | | |
| Delay (s) | 0.7 | | | 0.2 | | |
| Level of Service | A | | | A | | |
| Approach Delay (s) | 0.7 | | | 0.2 | 0.0 | |
| Approach LOS | A | | | A | A | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | 0.4 | | HCM 2000 Level of Service | | A |
| HCM 2000 Volume to Capacity ratio | | 0.46 | | | | |
| Actuated Cycle Length (s) | | 110.0 | | Sum of lost time (s) | | 11.0 |
| Intersection Capacity Utilization | | 41.4% | | ICU Level of Service | | A |
| Analysis Period (min) | | 15 | | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

2: Druid Lake Park Dr & Eutaw Pl

12/20/2021



| Movement | EBL | EBT | WBT | WBR | SWL | SWR |
|-----------------------------------|-------|------|-------|------|---------------------------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 666 | 0 | 47 | 0 | 0 | 954 |
| Future Volume (vph) | 666 | 0 | 47 | 0 | 0 | 954 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.4 | | 5.6 | | | 5.4 |
| Lane Util. Factor | 1.00 | | 0.95 | | | 0.76 |
| Frt | 1.00 | | 1.00 | | | 0.85 |
| Flt Protected | 0.95 | | 1.00 | | | 1.00 |
| Satd. Flow (prot) | 1770 | | 3539 | | | 3610 |
| Flt Permitted | 0.95 | | 1.00 | | | 1.00 |
| Satd. Flow (perm) | 1770 | | 3539 | | | 3610 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 724 | 0 | 51 | 0 | 0 | 1037 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 204 |
| Lane Group Flow (vph) | 724 | 0 | 51 | 0 | 0 | 833 |
| Turn Type | Prot | | NA | | | Prot |
| Protected Phases | 2 | | 8 | | | 6 |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 144.6 | | 24.4 | | | 144.6 |
| Effective Green, g (s) | 144.6 | | 24.4 | | | 144.6 |
| Actuated g/C Ratio | 0.80 | | 0.14 | | | 0.80 |
| Clearance Time (s) | 5.4 | | 5.6 | | | 5.4 |
| Vehicle Extension (s) | 0.2 | | 5.0 | | | 3.0 |
| Lane Grp Cap (vph) | 1421 | | 479 | | | 2900 |
| v/s Ratio Prot | c0.41 | | c0.01 | | | 0.23 |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 0.51 | | 0.11 | | | 0.29 |
| Uniform Delay, d1 | 5.9 | | 68.2 | | | 4.5 |
| Progression Factor | 1.00 | | 1.00 | | | 1.00 |
| Incremental Delay, d2 | 1.3 | | 0.4 | | | 0.2 |
| Delay (s) | 7.2 | | 68.7 | | | 4.8 |
| Level of Service | A | | E | | | A |
| Approach Delay (s) | | 7.2 | 68.7 | | 4.8 | |
| Approach LOS | | A | E | | A | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 7.5 | | HCM 2000 Level of Service | A |
| HCM 2000 Volume to Capacity ratio | | | 0.45 | | | |
| Actuated Cycle Length (s) | | | 180.0 | | Sum of lost time (s) | 11.0 |
| Intersection Capacity Utilization | | | 46.9% | | ICU Level of Service | A |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

3: Madison Ave & Druid Lake Park Dr & Swann Dr

12/20/2021



| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SBL2 | SBL | SBR | NWL | NWR |
|------------------------|------|-------|------|------|--------|------|------|------|------|------|------|
| Lane Configurations | | | | | | | | | | | |
| Traffic Volume (vph) | 8 | 660 | 13 | 88 | 860 | 53 | 6 | 13 | 5 | 0 | 0 |
| Future Volume (vph) | 8 | 660 | 13 | 88 | 860 | 53 | 6 | 13 | 5 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.0 | 5.6 | | | 6.6 | | | 5.6 | | | |
| Lane Util. Factor | 1.00 | 1.00 | | | 0.91 | | | 1.00 | | | |
| Frt | 1.00 | 1.00 | | | 0.99 | | | 0.97 | | | |
| Flt Protected | 0.95 | 1.00 | | | 1.00 | | | 0.96 | | | |
| Satd. Flow (prot) | 1770 | 1857 | | | 5023 | | | 1744 | | | |
| Flt Permitted | 0.95 | 1.00 | | | 0.77 | | | 0.96 | | | |
| Satd. Flow (perm) | 1770 | 1857 | | | 3889 | | | 1744 | | | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 9 | 717 | 14 | 96 | 935 | 58 | 7 | 14 | 5 | 0 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 23 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 9 | 731 | 0 | 0 | 1085 | 0 | 0 | 3 | 0 | 0 | 0 |
| Turn Type | Prot | NA | | Prot | NA | | Perm | Prot | | | |
| Protected Phases | 1 | 6 | | 5 | 2 | | | 4 | | | |
| Permitted Phases | | | | | | | 8 | | | | |
| Actuated Green, G (s) | 1.6 | 119.8 | | | 112.2 | | | 19.0 | | | |
| Effective Green, g (s) | 1.6 | 119.8 | | | 112.2 | | | 19.0 | | | |
| Actuated g/C Ratio | 0.01 | 0.80 | | | 0.75 | | | 0.13 | | | |
| Clearance Time (s) | 5.0 | 5.6 | | | 6.6 | | | 5.6 | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | | 3.0 | | | 3.0 | | | |
| Lane Grp Cap (vph) | 18 | 1483 | | | 2908 | | | 220 | | | |
| v/s Ratio Prot | 0.01 | c0.39 | | | | | | | | | |
| v/s Ratio Perm | | | | | 0.28 | | | 0.00 | | | |
| v/c Ratio | 0.50 | 0.49 | | | 6.86dl | | | 0.01 | | | |
| Uniform Delay, d1 | 73.8 | 5.0 | | | 6.6 | | | 57.3 | | | |
| Progression Factor | 1.00 | 1.00 | | | 1.00 | | | 1.00 | | | |
| Incremental Delay, d2 | 20.2 | 1.2 | | | 0.1 | | | 0.0 | | | |
| Delay (s) | 94.0 | 6.2 | | | 6.7 | | | 57.3 | | | |
| Level of Service | F | A | | | A | | | E | | | |
| Approach Delay (s) | | 7.3 | | | 6.7 | | | 57.3 | | 0.0 | |
| Approach LOS | | A | | | A | | | E | | A | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 7.6 | HCM 2000 Level of Service | A |
| HCM 2000 Volume to Capacity ratio | 0.45 | | |
| Actuated Cycle Length (s) | 150.0 | Sum of lost time (s) | 17.2 |
| Intersection Capacity Utilization | 78.3% | ICU Level of Service | D |
| Analysis Period (min) | 15 | | |

dl Defacto Left Lane. Recode with 1 though lane as a left lane.













dr Defacto Right Lane. Recode with 1 though lane as a right lane.

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

4: McCulloh St & Druid Lake Park Dr







12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SEL | SET | SER | NWL | NWT | NWR |
| Lane Configurations | | ↑↑ | | | ↑↑ | | | | | | ↑↑↑ | ↑ |
| Traffic Volume (vph) | 0 | 653 | 0 | 0 | 254 | 0 | 0 | 0 | 0 | 0 | 144 | 28 |
| Future Volume (vph) | 0 | 653 | 0 | 0 | 254 | 0 | 0 | 0 | 0 | 0 | 144 | 28 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 5.6 | | | 5.6 | | | | | | 6.6 | 6.6 |
| Lane Util. Factor | | 0.95 | | | 0.95 | | | | | | 0.91 | 1.00 |
| Frt | | 1.00 | | | 1.00 | | | | | | 1.00 | 0.85 |
| Flt Protected | | 1.00 | | | 1.00 | | | | | | 1.00 | 1.00 |
| Satd. Flow (prot) | | 3539 | | | 3539 | | | | | | 5085 | 1583 |
| Flt Permitted | | 1.00 | | | 1.00 | | | | | | 1.00 | 1.00 |
| Satd. Flow (perm) | | 3539 | | | 3539 | | | | | | 5085 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 710 | 0 | 0 | 276 | 0 | 0 | 0 | 0 | 0 | 157 | 30 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 27 |
| Lane Group Flow (vph) | 0 | 710 | 0 | 0 | 276 | 0 | 0 | 0 | 0 | 0 | 157 | 3 |
| Turn Type | | NA | | | NA | | | | | | NA | Perm |
| Protected Phases | | 2 | | | 6 | | | | | | 8 | |
| Permitted Phases | | | | | | | | | | | | 8 |
| Actuated Green, G (s) | | 147.5 | | | 147.5 | | | | | | 20.3 | 20.3 |
| Effective Green, g (s) | | 147.5 | | | 147.5 | | | | | | 20.3 | 20.3 |
| Actuated g/C Ratio | | 0.82 | | | 0.82 | | | | | | 0.11 | 0.11 |
| Clearance Time (s) | | 5.6 | | | 5.6 | | | | | | 6.6 | 6.6 |
| Vehicle Extension (s) | | 3.0 | | | 3.0 | | | | | | 3.0 | 3.0 |
| Lane Grp Cap (vph) | | 2900 | | | 2900 | | | | | | 573 | 178 |
| v/s Ratio Prot | | c0.20 | | | 0.08 | | | | | | c0.03 | |
| v/s Ratio Perm | | | | | | | | | | | | 0.00 |
| v/c Ratio | | 0.24 | | | 0.10 | | | | | | 0.27 | 0.02 |
| Uniform Delay, d1 | | 3.7 | | | 3.2 | | | | | | 73.1 | 71.0 |
| Progression Factor | | 0.57 | | | 1.00 | | | | | | 1.00 | 1.00 |
| Incremental Delay, d2 | | 0.2 | | | 0.1 | | | | | | 0.3 | 0.0 |
| Delay (s) | | 2.3 | | | 3.2 | | | | | | 73.4 | 71.0 |
| Level of Service | | A | | | A | | | | | | E | E |
| Approach Delay (s) | | 2.3 | | | 3.2 | | | 0.0 | | | 73.0 | |
| Approach LOS | | A | | | A | | | A | | | E | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | 13.8 | | | HCM 2000 Level of Service | | | B | | | | |
| HCM 2000 Volume to Capacity ratio | | 0.25 | | | | | | | | | | |
| Actuated Cycle Length (s) | | 180.0 | | | Sum of lost time (s) | | | 12.2 | | | | |
| Intersection Capacity Utilization | | 32.4% | | | ICU Level of Service | | | A | | | | |
| Analysis Period (min) | | 15 | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

5: Fulton Ave & Druid Hill Ave



















12/20/2021

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Movement | SET | SER | NWL | NWT | NEL | NER |
| Lane Configurations | ↑↑↑ | | ↑↑ | | | ↑↑ |
| Traffic Volume (vph) | 717 | 0 | 254 | 0 | 0 | 254 |
| Future Volume (vph) | 717 | 0 | 254 | 0 | 0 | 254 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.6 | | 6.6 | | | 6.6 |
| Lane Util. Factor | 0.91 | | 0.97 | | | 0.88 |
| Frt | 1.00 | | 1.00 | | | 0.85 |
| Flt Protected | 1.00 | | 0.95 | | | 1.00 |
| Satd. Flow (prot) | 5085 | | 3433 | | | 2787 |
| Flt Permitted | 1.00 | | 0.95 | | | 1.00 |
| Satd. Flow (perm) | 5085 | | 3433 | | | 2787 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 779 | 0 | 276 | 0 | 0 | 276 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 245 |
| Lane Group Flow (vph) | 779 | 0 | 276 | 0 | 0 | 31 |
| Turn Type | NA | | Prot | | | Over |
| Protected Phases | 2 | | 8 | | | 8 |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 147.5 | | 20.3 | | | 20.3 |
| Effective Green, g (s) | 147.5 | | 20.3 | | | 20.3 |
| Actuated g/C Ratio | 0.82 | | 0.11 | | | 0.11 |
| Clearance Time (s) | 5.6 | | 6.6 | | | 6.6 |
| Vehicle Extension (s) | 3.0 | | 3.0 | | | 3.0 |
| Lane Grp Cap (vph) | 4166 | | 387 | | | 314 |
| v/s Ratio Prot | c0.15 | | c0.08 | | | 0.01 |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 0.19 | | 0.71 | | | 0.10 |
| Uniform Delay, d1 | 3.5 | | 77.0 | | | 71.6 |
| Progression Factor | 1.00 | | 1.16 | | | 1.00 |
| Incremental Delay, d2 | 0.1 | | 6.1 | | | 0.1 |
| Delay (s) | 3.6 | | 95.8 | | | 71.8 |
| Level of Service | A | | F | | | E |
| Approach Delay (s) | 3.6 | | | 95.8 | 71.8 | |
| Approach LOS | A | | | F | E | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 36.8 | | HCM 2000 Level of Service | D |
| HCM 2000 Volume to Capacity ratio | | | 0.25 | | | |
| Actuated Cycle Length (s) | | | 180.0 | | Sum of lost time (s) | 12.2 |
| Intersection Capacity Utilization | | | 34.1% | | ICU Level of Service | A |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

7: Gwynns Falls Pkwy & Auchentoroly Terr


12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| Lane Configurations | |  | |  |  | | |  |  | |  | |
| Traffic Volume (vph) | 13 | 427 | 17 | 277 | 476 | 2 | 33 | 41 | 300 | 0 | 11 | 8 |
| Future Volume (vph) | 13 | 427 | 17 | 277 | 476 | 2 | 33 | 41 | 300 | 0 | 11 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 6.0 | | 5.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Lane Util. Factor | | 0.91 | | 0.97 | 0.91 | | | 0.91 | 0.91 | | 1.00 | |
| Frt | | 0.99 | | 1.00 | 1.00 | | | 0.90 | 0.85 | | 0.94 | |
| Flt Protected | | 1.00 | | 0.95 | 1.00 | | | 0.99 | 1.00 | | 1.00 | |
| Satd. Flow (prot) | | 5050 | | 3433 | 5082 | | | 3028 | 1441 | | 1755 | |
| Flt Permitted | | 0.92 | | 0.95 | 1.00 | | | 0.90 | 1.00 | | 1.00 | |
| Satd. Flow (perm) | | 4650 | | 3433 | 5082 | | | 2757 | 1441 | | 1755 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 14 | 464 | 18 | 301 | 517 | 2 | 36 | 45 | 326 | 0 | 12 | 9 |
| RTOR Reduction (vph) | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 |
| Lane Group Flow (vph) | 0 | 493 | 0 | 301 | 519 | 0 | 0 | 244 | 163 | 0 | 13 | 0 |
| Turn Type | Perm | NA | | Prot | NA | | Perm | NA | Perm | | NA | |
| Protected Phases | | 6 | | 5 | 2 | | | 4 | | | 8 | |
| Permitted Phases | 6 | | | | | | 4 | | 4 | 8 | | |
| Actuated Green, G (s) | | 60.0 | | 14.9 | 79.9 | | | 18.1 | 18.1 | | 18.1 | |
| Effective Green, g (s) | | 60.0 | | 14.9 | 79.9 | | | 18.1 | 18.1 | | 18.1 | |
| Actuated g/C Ratio | | 0.55 | | 0.14 | 0.73 | | | 0.16 | 0.16 | | 0.16 | |
| Clearance Time (s) | | 6.0 | | 5.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Vehicle Extension (s) | | 3.0 | | 3.0 | 3.0 | | | 3.0 | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | | 2536 | | 465 | 3691 | | | 453 | 237 | | 288 | |
| v/s Ratio Prot | | | | c0.09 | 0.10 | | | | | | 0.01 | |
| v/s Ratio Perm | | c0.11 | | | | | | 0.09 | c0.11 | | | |
| v/c Ratio | | 0.19 | | 0.65 | 0.14 | | | 0.54 | 0.69 | | 0.05 | |
| Uniform Delay, d1 | | 12.7 | | 45.1 | 4.6 | | | 42.1 | 43.3 | | 38.7 | |
| Progression Factor | | 1.66 | | 1.00 | 1.00 | | | 1.00 | 1.00 | | 1.00 | |
| Incremental Delay, d2 | | 0.2 | | 3.1 | 0.1 | | | 1.2 | 8.0 | | 0.1 | |
| Delay (s) | | 21.3 | | 48.2 | 4.7 | | | 43.4 | 51.3 | | 38.8 | |
| Level of Service | | C | | D | A | | | D | D | | D | |
| Approach Delay (s) | | 21.3 | | | 20.6 | | | 46.5 | | | 38.8 | |
| Approach LOS | | C | | | C | | | D | | | D | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 27.1 | | | HCM 2000 Level of Service | | | | C | | |
| HCM 2000 Volume to Capacity ratio | | | 0.36 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | Sum of lost time (s) | | | 17.0 | | | |
| Intersection Capacity Utilization | | | 42.1% | | | ICU Level of Service | | | A | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

9: Liberty Heights Ave/Greenspring Ave & Auchentoroly Terr













12/20/2021

| |  | | | | | | | | | | | |
|-----------------------------------|--|------|-------|-------|-------|---------------------------|------|-------|-------|--------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | ↑↑ | | ↔ | ↑↑ | | | ↔ | ↔ | | ↔ | |
| Traffic Volume (vph) | 0 | 249 | 5 | 152 | 307 | 58 | 9 | 41 | 167 | 41 | 81 | 1 |
| Future Volume (vph) | 0 | 249 | 5 | 152 | 307 | 58 | 9 | 41 | 167 | 41 | 81 | 1 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 6.0 | | 6.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Lane Util. Factor | | 0.95 | | 0.97 | 0.95 | | | 0.91 | 0.91 | | 0.95 | |
| Frt | | 1.00 | | 1.00 | 0.98 | | | 0.91 | 0.85 | | 1.00 | |
| Flt Protected | | 1.00 | | 0.95 | 1.00 | | | 1.00 | 1.00 | | 0.98 | |
| Satd. Flow (prot) | | 3530 | | 3433 | 3455 | | | 3063 | 1441 | | 3477 | |
| Flt Permitted | | 1.00 | | 0.95 | 1.00 | | | 0.92 | 1.00 | | 0.81 | |
| Satd. Flow (perm) | | 3530 | | 3433 | 3455 | | | 2838 | 1441 | | 2850 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 271 | 5 | 165 | 334 | 63 | 10 | 45 | 182 | 45 | 88 | 1 |
| RTOR Reduction (vph) | 0 | 1 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 275 | 0 | 165 | 390 | 0 | 0 | 146 | 91 | 0 | 134 | 0 |
| Turn Type | | NA | | Prot | NA | | Perm | NA | pm+ov | custom | NA | |
| Protected Phases | | 6 | | 1 | 2 | | | 4 | 1 | | | |
| Permitted Phases | | | | | | | 4 | | 4 | 8 | 8 | |
| Actuated Green, G (s) | | 70.4 | | 10.6 | 87.0 | | | 11.0 | 21.6 | | 11.0 | |
| Effective Green, g (s) | | 70.4 | | 10.6 | 87.0 | | | 11.0 | 21.6 | | 11.0 | |
| Actuated g/C Ratio | | 0.64 | | 0.10 | 0.79 | | | 0.10 | 0.20 | | 0.10 | |
| Clearance Time (s) | | 6.0 | | 6.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Vehicle Extension (s) | | 3.0 | | 3.0 | 3.0 | | | 3.0 | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | | 2259 | | 330 | 2732 | | | 283 | 361 | | 285 | |
| v/s Ratio Prot | | 0.08 | | c0.05 | c0.11 | | | | 0.02 | | | |
| v/s Ratio Perm | | | | | | | | c0.05 | 0.04 | | 0.05 | |
| v/c Ratio | | 0.12 | | 0.50 | 0.14 | | | 0.52 | 0.25 | | 0.47 | |
| Uniform Delay, d1 | | 7.7 | | 47.2 | 2.7 | | | 47.0 | 37.4 | | 46.7 | |
| Progression Factor | | 0.38 | | 0.98 | 0.93 | | | 0.55 | 0.71 | | 1.00 | |
| Incremental Delay, d2 | | 0.1 | | 1.2 | 0.1 | | | 1.6 | 0.4 | | 1.2 | |
| Delay (s) | | 3.0 | | 47.3 | 2.6 | | | 27.5 | 26.8 | | 48.0 | |
| Level of Service | | A | | D | A | | | C | C | | D | |
| Approach Delay (s) | | 3.0 | | | 15.8 | | | 27.2 | | | 48.0 | |
| Approach LOS | | A | | | B | | | C | | | D | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 18.7 | | | HCM 2000 Level of Service | | | B | | | |
| HCM 2000 Volume to Capacity ratio | | | 0.23 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | Sum of lost time (s) | | | 18.0 | | | |
| Intersection Capacity Utilization | | | 36.8% | | | ICU Level of Service | | | A | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

10: Reisterstown Rd & Liberty Heights Ave

12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SBL2 | SBL | SBR | NWL | NWR | NWR2 |
| Lane Configurations | | ↑↑ | ↑ | ↑ | ↑↑ | | | ↑↑ | ↑ | ↑ | ↑↑ | |
| Traffic Volume (vph) | 0 | 175 | 257 | 52 | 184 | 2 | 10 | 189 | 15 | 118 | 169 | 12 |
| Future Volume (vph) | 0 | 175 | 257 | 52 | 184 | 2 | 10 | 189 | 15 | 118 | 169 | 12 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 5.0 | 4.0 | 5.0 | 5.0 | | | 4.0 | 4.0 | 4.0 | 4.0 | |
| Lane Util. Factor | | 0.95 | 1.00 | 1.00 | 0.95 | | | 0.97 | 1.00 | 1.00 | 0.88 | |
| Frt | | 1.00 | 0.85 | 1.00 | 1.00 | | | 1.00 | 0.85 | 1.00 | 0.85 | |
| Flt Protected | | 1.00 | 1.00 | 0.95 | 1.00 | | | 0.95 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | | 3539 | 1583 | 1770 | 3534 | | | 3433 | 1583 | 1770 | 2787 | |
| Flt Permitted | | 1.00 | 1.00 | 0.59 | 1.00 | | | 0.94 | 1.00 | 1.00 | 1.00 | |
| Satd. Flow (perm) | | 3539 | 1583 | 1101 | 3534 | | | 3399 | 1583 | 1863 | 2787 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 190 | 279 | 57 | 200 | 2 | 11 | 205 | 16 | 128 | 184 | 13 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 5 | 0 | 4 | 0 |
| Lane Group Flow (vph) | 0 | 190 | 279 | 57 | 201 | 0 | 0 | 216 | 11 | 128 | 193 | 0 |
| Turn Type | | NA | pm+ov | custom | NA | | Perm | Prot | Perm | pm+pt | Prot | |
| Protected Phases | | 4 | 1 | | | | | 6 | | 1 | 2 | |
| Permitted Phases | | | 4 | 8 | 8 | | 6 | | 6 | 2 | | |
| Actuated Green, G (s) | | 11.9 | 24.4 | 11.9 | 11.9 | | | 72.6 | 72.6 | 89.1 | 89.1 | |
| Effective Green, g (s) | | 11.9 | 24.4 | 11.9 | 11.9 | | | 72.6 | 72.6 | 89.1 | 89.1 | |
| Actuated g/C Ratio | | 0.11 | 0.22 | 0.11 | 0.11 | | | 0.66 | 0.66 | 0.81 | 0.81 | |
| Clearance Time (s) | | 5.0 | 4.0 | 5.0 | 5.0 | | | 4.0 | 4.0 | 4.0 | 4.0 | |
| Vehicle Extension (s) | | 3.0 | 3.0 | 3.0 | 3.0 | | | 3.0 | 3.0 | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | | 382 | 351 | 119 | 382 | | | 2243 | 1044 | 1509 | 2257 | |
| v/s Ratio Prot | | 0.05 | c0.09 | | | | | c0.06 | 0.01 | 0.01 | 0.07 | |
| v/s Ratio Perm | | | 0.09 | 0.05 | 0.06 | | | | 0.01 | 0.06 | | |
| v/c Ratio | | 0.50 | 0.79 | 0.48 | 0.53 | | | 0.10 | 0.01 | 0.08 | 0.09 | |
| Uniform Delay, d1 | | 46.2 | 40.4 | 46.1 | 46.4 | | | 6.8 | 6.4 | 2.1 | 2.1 | |
| Progression Factor | | 1.00 | 1.00 | 0.94 | 0.94 | | | 1.01 | 1.00 | 1.00 | 1.00 | |
| Incremental Delay, d2 | | 1.0 | 11.7 | 3.0 | 1.3 | | | 0.1 | 0.0 | 0.0 | 0.1 | |
| Delay (s) | | 47.3 | 52.2 | 46.3 | 44.7 | | | 6.9 | 6.4 | 2.2 | 2.2 | |
| Level of Service | | D | D | D | D | | | A | A | A | A | |
| Approach Delay (s) | | 50.2 | | | 45.0 | | | 6.9 | | 2.2 | | |
| Approach LOS | | D | | | D | | | A | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 29.2 | | | HCM 2000 Level of Service | | | | C | | |
| HCM 2000 Volume to Capacity ratio | | | 0.27 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | Sum of lost time (s) | | | | 13.0 | | |
| Intersection Capacity Utilization | | | 37.2% | | | ICU Level of Service | | | | A | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

11: Reisterstown Rd & Anoka Ave

12/20/2021



| Movement | WBR2 | NBL | SEL | SER |
|-----------------------------------|--------|-------|---------------------------|-------|
| Lane Configurations | FF | FF | FF | FF |
| Traffic Volume (vph) | 317 | 171 | 254 | 214 |
| Future Volume (vph) | 317 | 171 | 254 | 214 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.88 | 0.97 | 0.97 | 0.88 |
| Frt | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 1.00 | 0.95 | 0.95 | 1.00 |
| Satd. Flow (prot) | 2787 | 3433 | 3433 | 2787 |
| Flt Permitted | 1.00 | 0.95 | 0.95 | 1.00 |
| Satd. Flow (perm) | 2787 | 3433 | 3433 | 2787 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 345 | 186 | 276 | 233 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 345 | 186 | 276 | 233 |
| Turn Type | custom | Prot | Prot | pt+ov |
| Protected Phases | 2 3 4 | 2 | 4 | 2 4 |
| Permitted Phases | | | | |
| Actuated Green, G (s) | 110.0 | 56.0 | 24.0 | 84.0 |
| Effective Green, g (s) | 103.0 | 56.0 | 24.0 | 84.0 |
| Actuated g/C Ratio | 0.94 | 0.51 | 0.22 | 0.76 |
| Clearance Time (s) | | 4.0 | 4.0 | |
| Vehicle Extension (s) | | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | 2609 | 1747 | 749 | 2128 |
| v/s Ratio Prot | c0.12 | 0.05 | c0.08 | 0.08 |
| v/s Ratio Perm | | | | |
| v/c Ratio | 0.13 | 0.11 | 0.37 | 0.11 |
| Uniform Delay, d1 | 0.3 | 14.0 | 36.6 | 3.4 |
| Progression Factor | 1.00 | 0.96 | 1.00 | 1.00 |
| Incremental Delay, d2 | 0.1 | 0.1 | 1.4 | 0.1 |
| Delay (s) | 0.4 | 13.6 | 38.0 | 3.5 |
| Level of Service | A | B | D | A |
| Approach Delay (s) | | | 22.2 | |
| Approach LOS | | | C | |
| Intersection Summary | | | | |
| HCM 2000 Control Delay | | 13.4 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | | 0.20 | | |
| Actuated Cycle Length (s) | | 110.0 | Sum of lost time (s) | 15.0 |
| Intersection Capacity Utilization | | Err% | ICU Level of Service | H |
| Analysis Period (min) | | 15 | | |
| c Critical Lane Group | | | | |

DPLD Existing Traffic Volumes (PM)

HCM Signalized Intersection Capacity Analysis

1: Linden Ave & Druid Lake Park Dr

12/20/2021

| | → | ↘ | ↙ | ← | ↖ | ↗ |
|-----------------------------------|-------|------|-------|-------|---------------------------|------|
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | ↰ | | ↱ | ↰↱ | | |
| Traffic Volume (vph) | 968 | 30 | 0 | 1482 | 0 | 0 |
| Future Volume (vph) | 968 | 30 | 0 | 1482 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.0 | | | 4.0 | | |
| Lane Util. Factor | 1.00 | | | 0.95 | | |
| Frt | 1.00 | | | 1.00 | | |
| Flt Protected | 1.00 | | | 1.00 | | |
| Satd. Flow (prot) | 1855 | | | 3539 | | |
| Flt Permitted | 1.00 | | | 1.00 | | |
| Satd. Flow (perm) | 1855 | | | 3539 | | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 1052 | 33 | 0 | 1611 | 0 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 1085 | 0 | 0 | 1611 | 0 | 0 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 6 | | 5 | 2 | | |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 110.0 | | | 110.0 | | |
| Effective Green, g (s) | 110.0 | | | 110.0 | | |
| Actuated g/C Ratio | 1.00 | | | 1.00 | | |
| Clearance Time (s) | 5.0 | | | 4.0 | | |
| Vehicle Extension (s) | 3.0 | | | 3.0 | | |
| Lane Grp Cap (vph) | 1855 | | | 3539 | | |
| v/s Ratio Prot | c0.58 | | | 0.46 | | |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 0.58 | | | 0.46 | | |
| Uniform Delay, d1 | 0.0 | | | 0.0 | | |
| Progression Factor | 1.00 | | | 1.00 | | |
| Incremental Delay, d2 | 1.4 | | | 0.4 | | |
| Delay (s) | 1.4 | | | 0.4 | | |
| Level of Service | A | | | A | | |
| Approach Delay (s) | 1.4 | | | 0.4 | 0.0 | |
| Approach LOS | A | | | A | A | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 0.8 | | HCM 2000 Level of Service | A |
| HCM 2000 Volume to Capacity ratio | | | 0.65 | | | |
| Actuated Cycle Length (s) | | | 110.0 | | Sum of lost time (s) | 11.0 |
| Intersection Capacity Utilization | | | 56.9% | | ICU Level of Service | B |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

2: Druid Lake Park Dr & Eutaw Pl

12/20/2021



















| Movement | EBL | EBT | WBT | WBR | SWL | SWR |
|-----------------------------------|-------|------|-------|------|---------------------------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 918 | 0 | 123 | 0 | 0 | 1482 |
| Future Volume (vph) | 918 | 0 | 123 | 0 | 0 | 1482 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.4 | | 5.6 | | | 5.4 |
| Lane Util. Factor | 1.00 | | 0.95 | | | 0.76 |
| Frt | 1.00 | | 1.00 | | | 0.85 |
| Flt Protected | 0.95 | | 1.00 | | | 1.00 |
| Satd. Flow (prot) | 1770 | | 3539 | | | 3610 |
| Flt Permitted | 0.95 | | 1.00 | | | 1.00 |
| Satd. Flow (perm) | 1770 | | 3539 | | | 3610 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 998 | 0 | 134 | 0 | 0 | 1611 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 355 |
| Lane Group Flow (vph) | 998 | 0 | 134 | 0 | 0 | 1256 |
| Turn Type | Prot | | NA | | | Prot |
| Protected Phases | 2 | | 8 | | | 6 |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 104.6 | | 34.4 | | | 104.6 |
| Effective Green, g (s) | 104.6 | | 34.4 | | | 104.6 |
| Actuated g/C Ratio | 0.70 | | 0.23 | | | 0.70 |
| Clearance Time (s) | 5.4 | | 5.6 | | | 5.4 |
| Vehicle Extension (s) | 0.2 | | 5.0 | | | 3.0 |
| Lane Grp Cap (vph) | 1234 | | 811 | | | 2517 |
| v/s Ratio Prot | c0.56 | | c0.04 | | | 0.35 |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 0.81 | | 0.17 | | | 0.50 |
| Uniform Delay, d1 | 15.8 | | 46.3 | | | 10.5 |
| Progression Factor | 0.18 | | 1.00 | | | 1.00 |
| Incremental Delay, d2 | 4.3 | | 0.4 | | | 0.7 |
| Delay (s) | 7.1 | | 46.7 | | | 11.2 |
| Level of Service | A | | D | | | B |
| Approach Delay (s) | | 7.1 | 46.7 | | 11.2 | |
| Approach LOS | | A | D | | B | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 11.5 | | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | | | 0.65 | | | |
| Actuated Cycle Length (s) | | | 150.0 | | Sum of lost time (s) | 11.0 |
| Intersection Capacity Utilization | | | 71.4% | | ICU Level of Service | C |
| Analysis Period (min) | | | 15 | | | |

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Madison Ave & Druid Lake Park Dr & Swann Dr













12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |
|---|---|---|---|---|---|---|--|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SBL2 | SBL | SBR | NWL | NWR |
| Lane Configurations |  |  | | |  |  | |  | | | |
| Traffic Volume (vph) | 15 | 902 | 24 | 134 | 1434 | 37 | 16 | 30 | 15 | 0 | 0 |
| Future Volume (vph) | 15 | 902 | 24 | 134 | 1434 | 37 | 16 | 30 | 15 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.0 | 6.6 | | | 6.6 | | | 5.6 | | | |
| Lane Util. Factor | 1.00 | 1.00 | | | 0.91 | | | 1.00 | | | |
| Frt | 1.00 | 1.00 | | | 1.00 | | | 0.97 | | | |
| Flt Protected | 0.95 | 1.00 | | | 1.00 | | | 0.96 | | | |
| Satd. Flow (prot) | 1770 | 1856 | | | 5047 | | | 1736 | | | |
| Flt Permitted | 0.95 | 1.00 | | | 0.69 | | | 0.96 | | | |
| Satd. Flow (perm) | 1770 | 1856 | | | 3497 | | | 1736 | | | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 16 | 980 | 26 | 146 | 1559 | 40 | 17 | 33 | 16 | 0 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 55 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 16 | 1006 | 0 | 0 | 1743 | 0 | 0 | 11 | 0 | 0 | 0 |
| Turn Type | Prot | NA | | Prot | NA | | Perm | Prot | | | |
| Protected Phases | 1 | 6 | | 5 | 2 | | | 4 | | | |
| Permitted Phases | | | | | | | 8 | | | | |
| Actuated Green, G (s) | 3.3 | 118.8 | | | 110.5 | | | 19.0 | | | |
| Effective Green, g (s) | 3.3 | 118.8 | | | 110.5 | | | 19.0 | | | |
| Actuated g/C Ratio | 0.02 | 0.79 | | | 0.74 | | | 0.13 | | | |
| Clearance Time (s) | 5.0 | 6.6 | | | 6.6 | | | 5.6 | | | |
| Vehicle Extension (s) | 3.0 | 3.0 | | | 3.0 | | | 3.0 | | | |
| Lane Grp Cap (vph) | 38 | 1469 | | | 2576 | | | 219 | | | |
| v/s Ratio Prot | 0.01 | c0.54 | | | | | | | | | |
| v/s Ratio Perm | | | | | 0.50 | | | 0.01 | | | |
| v/c Ratio | 0.42 | 0.68 | | | 24.33dl | | | 0.05 | | | |
| Uniform Delay, d1 | 72.4 | 7.1 | | | 10.4 | | | 57.6 | | | |
| Progression Factor | 1.45 | 0.99 | | | 1.10 | | | 1.00 | | | |
| Incremental Delay, d2 | 7.0 | 2.5 | | | 0.6 | | | 0.1 | | | |
| Delay (s) | 112.0 | 9.5 | | | 12.0 | | | 57.7 | | | |
| Level of Service | F | A | | | B | | | E | | | |
| Approach Delay (s) | | 11.1 | | | 12.0 | | | 57.7 | | 0.0 | |
| Approach LOS | | B | | | B | | | E | | A | |
| Intersection Summary | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 12.7 | | | HCM 2000 Level of Service | | | B | | |
| HCM 2000 Volume to Capacity ratio | | | 0.62 | | | | | | | | |
| Actuated Cycle Length (s) | | | 150.0 | | | Sum of lost time (s) | | | 17.2 | | |
| Intersection Capacity Utilization | | | 104.2% | | | ICU Level of Service | | | G | | |
| Analysis Period (min) | | | 15 | | | | | | | | |
| dl Defacto Left Lane. Recode with 1 though lane as a left lane. | | | | | | | | | | | |
| dr Defacto Right Lane. Recode with 1 though lane as a right lane. | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

4: McCulloh St & Druid Lake Park Dr







12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SEL | SET | SER | NWL | NWT | NWR |
| Lane Configurations | | ↑↑ | | | ↑↑ | | | | | | ↑↑↑ | ↑ |
| Traffic Volume (vph) | 0 | 894 | 0 | 0 | 452 | 0 | 0 | 0 | 0 | 0 | 369 | 47 |
| Future Volume (vph) | 0 | 894 | 0 | 0 | 452 | 0 | 0 | 0 | 0 | 0 | 369 | 47 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 5.6 | | | 5.6 | | | | | | 6.6 | 6.6 |
| Lane Util. Factor | | 0.95 | | | 0.95 | | | | | | 0.91 | 1.00 |
| Frt | | 1.00 | | | 1.00 | | | | | | 1.00 | 0.85 |
| Flt Protected | | 1.00 | | | 1.00 | | | | | | 1.00 | 1.00 |
| Satd. Flow (prot) | | 3539 | | | 3539 | | | | | | 5085 | 1583 |
| Flt Permitted | | 1.00 | | | 1.00 | | | | | | 1.00 | 1.00 |
| Satd. Flow (perm) | | 3539 | | | 3539 | | | | | | 5085 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 972 | 0 | 0 | 491 | 0 | 0 | 0 | 0 | 0 | 401 | 51 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 40 |
| Lane Group Flow (vph) | 0 | 972 | 0 | 0 | 491 | 0 | 0 | 0 | 0 | 0 | 401 | 11 |
| Turn Type | | NA | | | NA | | | | | | NA | Perm |
| Protected Phases | | 2 | | | 6 | | | | | | 8 | |
| Permitted Phases | | | | | | | | | | | | 8 |
| Actuated Green, G (s) | | 105.9 | | | 105.9 | | | | | | 31.9 | 31.9 |
| Effective Green, g (s) | | 105.9 | | | 105.9 | | | | | | 31.9 | 31.9 |
| Actuated g/C Ratio | | 0.71 | | | 0.71 | | | | | | 0.21 | 0.21 |
| Clearance Time (s) | | 5.6 | | | 5.6 | | | | | | 6.6 | 6.6 |
| Vehicle Extension (s) | | 3.0 | | | 3.0 | | | | | | 3.0 | 3.0 |
| Lane Grp Cap (vph) | | 2498 | | | 2498 | | | | | | 1081 | 336 |
| v/s Ratio Prot | | c0.27 | | | 0.14 | | | | | | c0.08 | |
| v/s Ratio Perm | | | | | | | | | | | | 0.01 |
| v/c Ratio | | 0.39 | | | 0.20 | | | | | | 0.37 | 0.03 |
| Uniform Delay, d1 | | 8.9 | | | 7.5 | | | | | | 50.5 | 46.8 |
| Progression Factor | | 0.73 | | | 0.91 | | | | | | 1.00 | 1.00 |
| Incremental Delay, d2 | | 0.4 | | | 0.0 | | | | | | 0.2 | 0.0 |
| Delay (s) | | 7.0 | | | 6.9 | | | | | | 50.7 | 46.9 |
| Level of Service | | A | | | A | | | | | | D | D |
| Approach Delay (s) | | 7.0 | | | 6.9 | | | 0.0 | | | 50.3 | |
| Approach LOS | | A | | | A | | | A | | | D | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | 17.2 | | | HCM 2000 Level of Service | | | B | | | | |
| HCM 2000 Volume to Capacity ratio | | 0.38 | | | | | | | | | | |
| Actuated Cycle Length (s) | | 150.0 | | | Sum of lost time (s) | | | 12.2 | | | | |
| Intersection Capacity Utilization | | 42.0% | | | ICU Level of Service | | | A | | | | |
| Analysis Period (min) | | 15 | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

5: Fulton Ave & Druid Hill Ave



















12/20/2021

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Movement | SET | SER | NWL | NWT | NEL | NER |
| Lane Configurations | ↑↑↑ | | ↑↑ | | | ↑↑ |
| Traffic Volume (vph) | 1041 | 0 | 452 | 0 | 0 | 382 |
| Future Volume (vph) | 1041 | 0 | 452 | 0 | 0 | 382 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.6 | | 6.6 | | | 6.6 |
| Lane Util. Factor | 0.91 | | 0.97 | | | 0.88 |
| Frt | 1.00 | | 1.00 | | | 0.85 |
| Flt Protected | 1.00 | | 0.95 | | | 1.00 |
| Satd. Flow (prot) | 5085 | | 3433 | | | 2787 |
| Flt Permitted | 1.00 | | 0.95 | | | 1.00 |
| Satd. Flow (perm) | 5085 | | 3433 | | | 2787 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 1132 | 0 | 491 | 0 | 0 | 415 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 169 |
| Lane Group Flow (vph) | 1132 | 0 | 491 | 0 | 0 | 246 |
| Turn Type | NA | | Prot | | | Over |
| Protected Phases | 2 | | 8 | | | 8 |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 105.9 | | 31.9 | | | 31.9 |
| Effective Green, g (s) | 105.9 | | 31.9 | | | 31.9 |
| Actuated g/C Ratio | 0.71 | | 0.21 | | | 0.21 |
| Clearance Time (s) | 5.6 | | 6.6 | | | 6.6 |
| Vehicle Extension (s) | 3.0 | | 3.0 | | | 3.0 |
| Lane Grp Cap (vph) | 3590 | | 730 | | | 592 |
| v/s Ratio Prot | c0.22 | | c0.14 | | | 0.09 |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 0.32 | | 0.67 | | | 0.42 |
| Uniform Delay, d1 | 8.3 | | 54.3 | | | 51.0 |
| Progression Factor | 1.00 | | 1.17 | | | 1.00 |
| Incremental Delay, d2 | 0.2 | | 2.4 | | | 0.5 |
| Delay (s) | 8.6 | | 65.8 | | | 51.5 |
| Level of Service | A | | E | | | D |
| Approach Delay (s) | 8.6 | | | 65.8 | 51.5 | |
| Approach LOS | A | | | E | D | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 31.1 | | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | | | 0.40 | | | |
| Actuated Cycle Length (s) | | | 150.0 | | Sum of lost time (s) | 12.2 |
| Intersection Capacity Utilization | | | 43.6% | | ICU Level of Service | A |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

7: Gwynns Falls Pkwy & Auchentoroly Terr


12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| Lane Configurations | |  | |  |  | | |  |  | |  | |
| Traffic Volume (vph) | 15 | 600 | 15 | 581 | 777 | 8 | 67 | 44 | 451 | 0 | 46 | 30 |
| Future Volume (vph) | 15 | 600 | 15 | 581 | 777 | 8 | 67 | 44 | 451 | 0 | 46 | 30 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 6.0 | | 5.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Lane Util. Factor | | 0.91 | | 0.97 | 0.91 | | | 0.91 | 0.91 | | 1.00 | |
| Frt | | 1.00 | | 1.00 | 1.00 | | | 0.90 | 0.85 | | 0.95 | |
| Flt Protected | | 1.00 | | 0.95 | 1.00 | | | 0.99 | 1.00 | | 1.00 | |
| Satd. Flow (prot) | | 5062 | | 3433 | 5077 | | | 3020 | 1441 | | 1763 | |
| Flt Permitted | | 0.91 | | 0.95 | 1.00 | | | 0.86 | 1.00 | | 1.00 | |
| Satd. Flow (perm) | | 4592 | | 3433 | 5077 | | | 2637 | 1441 | | 1763 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 16 | 652 | 16 | 632 | 845 | 9 | 73 | 48 | 490 | 0 | 50 | 33 |
| RTOR Reduction (vph) | 0 | 2 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 25 | 0 |
| Lane Group Flow (vph) | 0 | 682 | 0 | 632 | 853 | 0 | 0 | 366 | 245 | 0 | 58 | 0 |
| Turn Type | Perm | NA | | Prot | NA | | Perm | NA | Perm | | NA | |
| Protected Phases | | 6 | | 5 | 2 | | | 4 | | | 8 | |
| Permitted Phases | 6 | | | | | | 4 | | 4 | 8 | | |
| Actuated Green, G (s) | | 42.4 | | 25.5 | 72.9 | | | 25.1 | 25.1 | | 25.1 | |
| Effective Green, g (s) | | 42.4 | | 25.5 | 72.9 | | | 25.1 | 25.1 | | 25.1 | |
| Actuated g/C Ratio | | 0.39 | | 0.23 | 0.66 | | | 0.23 | 0.23 | | 0.23 | |
| Clearance Time (s) | | 6.0 | | 5.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Vehicle Extension (s) | | 3.0 | | 3.0 | 3.0 | | | 3.0 | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | | 1770 | | 795 | 3364 | | | 601 | 328 | | 402 | |
| v/s Ratio Prot | | | | c0.18 | 0.17 | | | | | | 0.03 | |
| v/s Ratio Perm | | c0.15 | | | | | | 0.14 | c0.17 | | | |
| v/c Ratio | | 0.39 | | 0.79 | 0.25 | | | 0.61 | 0.75 | | 0.15 | |
| Uniform Delay, d1 | | 24.4 | | 39.8 | 7.5 | | | 38.1 | 39.5 | | 33.9 | |
| Progression Factor | | 1.72 | | 1.00 | 1.00 | | | 1.00 | 1.00 | | 1.00 | |
| Incremental Delay, d2 | | 0.6 | | 5.5 | 0.2 | | | 1.8 | 9.0 | | 0.2 | |
| Delay (s) | | 42.5 | | 45.3 | 7.7 | | | 39.8 | 48.4 | | 34.1 | |
| Level of Service | | D | | D | A | | | D | D | | C | |
| Approach Delay (s) | | 42.5 | | | 23.7 | | | 43.3 | | | 34.1 | |
| Approach LOS | | D | | | C | | | D | | | C | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 32.7 | | | HCM 2000 Level of Service | | | | C | | |
| HCM 2000 Volume to Capacity ratio | | | 0.59 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | Sum of lost time (s) | | | 17.0 | | | |
| Intersection Capacity Utilization | | | 55.2% | | | ICU Level of Service | | | B | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

9: Liberty Heights Ave/Greenspring Ave & Auchentoroly Terr

12/20/2021


| |  | | | | | | | | | | | |
|-----------------------------------|--|------|-------|-------|-------|---------------------------|------|-------|-------|--------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | ↑↑ | | ↔↔ | ↑↑ | | | ↔↔ | ↔ | | ↔↔ | |
| Traffic Volume (vph) | 0 | 306 | 10 | 357 | 395 | 122 | 24 | 65 | 263 | 61 | 131 | 1 |
| Future Volume (vph) | 0 | 306 | 10 | 357 | 395 | 122 | 24 | 65 | 263 | 61 | 131 | 1 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 6.0 | | 6.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Lane Util. Factor | | 0.95 | | 0.97 | 0.95 | | | 0.91 | 0.91 | | 0.95 | |
| Frt | | 1.00 | | 1.00 | 0.96 | | | 0.91 | 0.85 | | 1.00 | |
| Flt Protected | | 1.00 | | 0.95 | 1.00 | | | 0.99 | 1.00 | | 0.98 | |
| Satd. Flow (prot) | | 3522 | | 3433 | 3414 | | | 3071 | 1441 | | 3482 | |
| Flt Permitted | | 1.00 | | 0.95 | 1.00 | | | 0.89 | 1.00 | | 0.74 | |
| Satd. Flow (perm) | | 3522 | | 3433 | 3414 | | | 2741 | 1441 | | 2610 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 333 | 11 | 388 | 429 | 133 | 26 | 71 | 286 | 66 | 142 | 1 |
| RTOR Reduction (vph) | 0 | 1 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 343 | 0 | 388 | 550 | 0 | 0 | 240 | 143 | 0 | 209 | 0 |
| Turn Type | | NA | | Prot | NA | | Perm | NA | pm+ov | custom | NA | |
| Protected Phases | | 6 | | 1 | 2 | | | 4 | 1 | | | |
| Permitted Phases | | | | | | | 4 | | 4 | 8 | 8 | |
| Actuated Green, G (s) | | 59.3 | | 17.8 | 83.1 | | | 14.9 | 32.7 | | 14.9 | |
| Effective Green, g (s) | | 59.3 | | 17.8 | 83.1 | | | 14.9 | 32.7 | | 14.9 | |
| Actuated g/C Ratio | | 0.54 | | 0.16 | 0.76 | | | 0.14 | 0.30 | | 0.14 | |
| Clearance Time (s) | | 6.0 | | 6.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Vehicle Extension (s) | | 3.0 | | 3.0 | 3.0 | | | 3.0 | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | | 1898 | | 555 | 2579 | | | 371 | 506 | | 353 | |
| v/s Ratio Prot | | 0.10 | | c0.11 | c0.16 | | | | 0.05 | | | |
| v/s Ratio Perm | | | | | | | | c0.09 | 0.05 | | 0.08 | |
| v/c Ratio | | 0.18 | | 0.70 | 0.21 | | | 0.65 | 0.28 | | 0.59 | |
| Uniform Delay, d1 | | 12.9 | | 43.6 | 3.9 | | | 45.1 | 29.7 | | 44.7 | |
| Progression Factor | | 1.22 | | 1.14 | 0.86 | | | 0.86 | 1.36 | | 1.00 | |
| Incremental Delay, d2 | | 0.2 | | 3.8 | 0.2 | | | 3.8 | 0.3 | | 2.7 | |
| Delay (s) | | 16.0 | | 53.3 | 3.5 | | | 42.4 | 40.6 | | 47.3 | |
| Level of Service | | B | | D | A | | | D | D | | D | |
| Approach Delay (s) | | 16.0 | | | 23.8 | | | 41.7 | | | 47.3 | |
| Approach LOS | | B | | | C | | | D | | | D | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 28.6 | | | HCM 2000 Level of Service | | | C | | | |
| HCM 2000 Volume to Capacity ratio | | | 0.38 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | Sum of lost time (s) | | | 18.0 | | | |
| Intersection Capacity Utilization | | | 50.6% | | | ICU Level of Service | | | A | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

10: Reisterstown Rd & Liberty Heights Ave

12/20/2021

| |  | | | | | | | | | | | |
|-----------------------------------|--|------|-------|--------|------|---------------------------|------|------|------|-------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SBL2 | SBL | SBR | NWL | NWR | NWR2 |
| Lane Configurations | | ↑↑ | ↑ | ↑ | ↑↑ | | | ↑↑ | ↑ | ↑ | ↑↑ | |
| Traffic Volume (vph) | 0 | 262 | 362 | 89 | 403 | 6 | 9 | 332 | 51 | 311 | 340 | 41 |
| Future Volume (vph) | 0 | 262 | 362 | 89 | 403 | 6 | 9 | 332 | 51 | 311 | 340 | 41 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 5.0 | 4.0 | 5.0 | 5.0 | | | 4.0 | 4.0 | 4.0 | 4.0 | |
| Lane Util. Factor | | 0.95 | 1.00 | 1.00 | 0.95 | | | 0.97 | 1.00 | 1.00 | 0.88 | |
| Frt | | 1.00 | 0.85 | 1.00 | 1.00 | | | 1.00 | 0.85 | 1.00 | 0.85 | |
| Flt Protected | | 1.00 | 1.00 | 0.95 | 1.00 | | | 0.95 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | | 3539 | 1583 | 1770 | 3531 | | | 3433 | 1583 | 1770 | 2787 | |
| Flt Permitted | | 1.00 | 1.00 | 0.49 | 1.00 | | | 0.94 | 1.00 | 1.00 | 1.00 | |
| Satd. Flow (perm) | | 3539 | 1583 | 907 | 3531 | | | 3405 | 1583 | 1863 | 2787 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 285 | 393 | 97 | 438 | 7 | 10 | 361 | 55 | 338 | 370 | 45 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 23 | 0 | 5 | 0 |
| Lane Group Flow (vph) | 0 | 285 | 393 | 97 | 443 | 0 | 0 | 371 | 32 | 338 | 410 | 0 |
| Turn Type | | NA | pm+ov | custom | NA | | Perm | Prot | Perm | pm+pt | Prot | |
| Protected Phases | | 4 | 1 | | | | | 6 | | 1 | 2 | |
| Permitted Phases | | | 4 | 8 | 8 | | 6 | | 6 | 2 | | |
| Actuated Green, G (s) | | 20.2 | 33.8 | 20.2 | 20.2 | | | 63.2 | 63.2 | 80.8 | 80.8 | |
| Effective Green, g (s) | | 20.2 | 33.8 | 20.2 | 20.2 | | | 63.2 | 63.2 | 80.8 | 80.8 | |
| Actuated g/C Ratio | | 0.18 | 0.31 | 0.18 | 0.18 | | | 0.57 | 0.57 | 0.73 | 0.73 | |
| Clearance Time (s) | | 5.0 | 4.0 | 5.0 | 5.0 | | | 4.0 | 4.0 | 4.0 | 4.0 | |
| Vehicle Extension (s) | | 3.0 | 3.0 | 3.0 | 3.0 | | | 3.0 | 3.0 | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | | 649 | 486 | 166 | 648 | | | 1956 | 909 | 1368 | 2047 | |
| v/s Ratio Prot | | 0.08 | c0.10 | | | | | | | 0.03 | 0.15 | |
| v/s Ratio Perm | | | 0.15 | 0.11 | 0.13 | | | 0.11 | 0.02 | c0.15 | | |
| v/c Ratio | | 0.44 | 0.81 | 0.58 | 0.68 | | | 0.19 | 0.03 | 0.25 | 0.20 | |
| Uniform Delay, d1 | | 39.9 | 35.1 | 41.1 | 41.9 | | | 11.2 | 10.2 | 4.9 | 4.5 | |
| Progression Factor | | 1.00 | 1.00 | 0.52 | 0.53 | | | 1.08 | 2.55 | 1.00 | 1.00 | |
| Incremental Delay, d2 | | 0.5 | 9.6 | 4.9 | 2.8 | | | 0.2 | 0.1 | 0.1 | 0.2 | |
| Delay (s) | | 40.3 | 44.7 | 26.2 | 25.0 | | | 12.3 | 26.0 | 5.0 | 4.8 | |
| Level of Service | | D | D | C | C | | | B | C | A | A | |
| Approach Delay (s) | | 42.9 | | | 25.2 | | | 14.0 | | 4.9 | | |
| Approach LOS | | D | | | C | | | B | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 21.8 | | | HCM 2000 Level of Service | | | | C | | |
| HCM 2000 Volume to Capacity ratio | | | 0.45 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | Sum of lost time (s) | | | | 13.0 | | |
| Intersection Capacity Utilization | | | 54.2% | | | ICU Level of Service | | | | A | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

11: Reisterstown Rd & Anoka Ave

12/20/2021












| Movement | WBR2 | NBL | SEL | SER |
|-----------------------------------|--------|-------|---------------------------|-------|
| Lane Configurations | FF | FF | FF | FF |
| Traffic Volume (vph) | 420 | 346 | 316 | 392 |
| Future Volume (vph) | 420 | 346 | 316 | 392 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.88 | 0.97 | 0.97 | 0.88 |
| Frt | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 1.00 | 0.95 | 0.95 | 1.00 |
| Satd. Flow (prot) | 2787 | 3433 | 3433 | 2787 |
| Flt Permitted | 1.00 | 0.95 | 0.95 | 1.00 |
| Satd. Flow (perm) | 2787 | 3433 | 3433 | 2787 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 457 | 376 | 343 | 426 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 457 | 376 | 343 | 426 |
| Turn Type | custom | Prot | Prot | pt+ov |
| Protected Phases | 2 3 4 | 2 | 4 | 2 4 |
| Permitted Phases | | | | |
| Actuated Green, G (s) | 110.0 | 27.0 | 53.0 | 84.0 |
| Effective Green, g (s) | 103.0 | 27.0 | 53.0 | 84.0 |
| Actuated g/C Ratio | 0.94 | 0.25 | 0.48 | 0.76 |
| Clearance Time (s) | | 4.0 | 4.0 | |
| Vehicle Extension (s) | | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | 2609 | 842 | 1654 | 2128 |
| v/s Ratio Prot | c0.16 | c0.11 | 0.10 | c0.15 |
| v/s Ratio Perm | | | | |
| v/c Ratio | 0.18 | 0.45 | 0.21 | 0.20 |
| Uniform Delay, d1 | 0.3 | 35.2 | 16.4 | 3.6 |
| Progression Factor | 1.00 | 0.88 | 1.00 | 1.00 |
| Incremental Delay, d2 | 0.1 | 1.7 | 0.3 | 0.2 |
| Delay (s) | 0.4 | 32.6 | 16.7 | 3.8 |
| Level of Service | A | C | B | A |
| Approach Delay (s) | | | 9.6 | |
| Approach LOS | | | A | |
| Intersection Summary | | | | |
| HCM 2000 Control Delay | | 12.4 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | | 0.28 | | |
| Actuated Cycle Length (s) | | 110.0 | Sum of lost time (s) | 15.0 |
| Intersection Capacity Utilization | | Err% | ICU Level of Service | H |
| Analysis Period (min) | | 15 | | |
| c Critical Lane Group | | | | |

HCM Signalized Intersection Capacity Analysis

1: Linden Ave & Druid Lake Park Dr

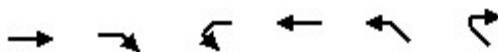
12/20/2021

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations |  | |  |  | | |
| Traffic Volume (vph) | 691 | 15 | 44 | 910 | 0 | 0 |
| Future Volume (vph) | 691 | 15 | 44 | 910 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.0 | | 4.5 | 5.0 | | |
| Lane Util. Factor | 1.00 | | 1.00 | 1.00 | | |
| Frt | 1.00 | | 1.00 | 1.00 | | |
| Flt Protected | 1.00 | | 0.95 | 1.00 | | |
| Satd. Flow (prot) | 1857 | | 1770 | 1863 | | |
| Flt Permitted | 1.00 | | 0.95 | 1.00 | | |
| Satd. Flow (perm) | 1857 | | 1770 | 1863 | | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 751 | 16 | 48 | 989 | 0 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 767 | 0 | 48 | 989 | 0 | 0 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 6 | | 5 | 2 | | |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 55.0 | | 5.5 | 90.0 | | |
| Effective Green, g (s) | 55.0 | | 5.5 | 90.0 | | |
| Actuated g/C Ratio | 0.46 | | 0.05 | 0.75 | | |
| Clearance Time (s) | 5.0 | | 4.5 | 5.0 | | |
| Lane Grp Cap (vph) | 851 | | 81 | 1397 | | |
| v/s Ratio Prot | c0.41 | | 0.03 | c0.53 | | |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 0.90 | | 0.59 | 0.71 | | |
| Uniform Delay, d1 | 30.0 | | 56.2 | 8.0 | | |
| Progression Factor | 0.09 | | 1.00 | 1.00 | | |
| Incremental Delay, d2 | 1.7 | | 28.0 | 3.1 | | |
| Delay (s) | 4.5 | | 84.2 | 11.0 | | |
| Level of Service | A | | F | B | | |
| Approach Delay (s) | 4.5 | | | 14.4 | 0.0 | |
| Approach LOS | A | | | B | A | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 10.2 | | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | | | 0.69 | | | |
| Actuated Cycle Length (s) | | | 120.0 | | Sum of lost time (s) | 17.5 |
| Intersection Capacity Utilization | | | 52.1% | | ICU Level of Service | A |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

2: Eutaw PI & Druid Lake Park Dr

12/20/2021
























| Movement | EBT | EBR | WBL | WBT | NWL | NWR |
|-----------------------------------|-------|------|-------|-------|---------------------------|------|
| Lane Configurations | ↩ | | ↩ | ↩ | ↩ | ↩ |
| Traffic Volume (vph) | 652 | 117 | 44 | 866 | 119 | 54 |
| Future Volume (vph) | 652 | 117 | 44 | 866 | 119 | 54 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.5 | | 4.5 | 5.5 | 5.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | 1.00 | 1.00 | |
| Frt | 0.98 | | 1.00 | 1.00 | 0.96 | |
| Flt Protected | 1.00 | | 0.95 | 1.00 | 0.97 | |
| Satd. Flow (prot) | 1825 | | 1770 | 1863 | 1725 | |
| Flt Permitted | 1.00 | | 0.95 | 1.00 | 0.97 | |
| Satd. Flow (perm) | 1825 | | 1770 | 1863 | 1725 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 709 | 127 | 48 | 941 | 129 | 59 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 836 | 0 | 48 | 941 | 188 | 0 |
| Turn Type | NA | | Prot | NA | Prot | |
| Protected Phases | 2 | | 1 | 6 | 8 | |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 52.5 | | 5.5 | 87.5 | 22.0 | |
| Effective Green, g (s) | 52.5 | | 5.5 | 87.5 | 22.0 | |
| Actuated g/C Ratio | 0.44 | | 0.05 | 0.73 | 0.18 | |
| Clearance Time (s) | 5.5 | | 4.5 | 5.5 | 5.0 | |
| Lane Grp Cap (vph) | 798 | | 81 | 1358 | 316 | |
| v/s Ratio Prot | c0.46 | | 0.03 | c0.51 | c0.11 | |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 1.05 | | 0.59 | 0.69 | 0.59 | |
| Uniform Delay, d1 | 33.8 | | 56.2 | 8.9 | 44.9 | |
| Progression Factor | 1.00 | | 0.85 | 1.13 | 1.00 | |
| Incremental Delay, d2 | 45.0 | | 20.2 | 2.0 | 8.0 | |
| Delay (s) | 78.8 | | 67.7 | 12.1 | 52.9 | |
| Level of Service | E | | E | B | D | |
| Approach Delay (s) | 78.8 | | | 14.8 | 52.9 | |
| Approach LOS | E | | | B | D | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 44.9 | | HCM 2000 Level of Service | D |
| HCM 2000 Volume to Capacity ratio | | | 0.89 | | | |
| Actuated Cycle Length (s) | | | 120.0 | | Sum of lost time (s) | 20.5 |
| Intersection Capacity Utilization | | | 64.2% | | ICU Level of Service | C |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

10: Reisterstown Rd & Liberty Heights Ave












12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SBL2 | SBL | SBR | NWL | NWR | NWR2 |
| Lane Configurations | |  |  | |  |  | |  |  |  |  |  |
| Traffic Volume (vph) | 0 | 186 | 257 | 52 | 169 | 320 | 244 | 189 | 35 | 118 | 168 | 7 |
| Future Volume (vph) | 0 | 186 | 257 | 52 | 169 | 320 | 244 | 189 | 35 | 118 | 168 | 7 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 5.0 | 5.0 | | 5.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | | 1.00 | 1.00 | | 0.95 | 1.00 | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 1.00 | 0.85 | | 1.00 | 0.85 | | 1.00 | 0.85 | 1.00 | 0.85 | 0.85 |
| Flt Protected | | 1.00 | 1.00 | | 0.99 | 1.00 | | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 1863 | 1583 | | 3498 | 1583 | | 1770 | 1583 | 1770 | 1583 | 1583 |
| Flt Permitted | | 1.00 | 1.00 | | 0.77 | 1.00 | | 0.56 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 1863 | 1583 | | 2735 | 1583 | | 1047 | 1583 | 1770 | 1583 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 202 | 279 | 57 | 184 | 348 | 265 | 205 | 38 | 128 | 183 | 8 |
| RTOR Reduction (vph) | 0 | 0 | 213 | 0 | 0 | 149 | 0 | 0 | 19 | 0 | 0 | 6 |
| Lane Group Flow (vph) | 0 | 202 | 66 | 0 | 241 | 199 | 0 | 470 | 19 | 128 | 183 | 2 |
| Turn Type | | NA | Perm | Perm | NA | pm+ov | Prot | Prot | Perm | Prot | Prot | Prot |
| Protected Phases | | 4 | | | 8 | 1 | 1 | 6 | | 5 | 2 | 2 |
| Permitted Phases | | | 4 | 8 | | 8 | | | 6 | | | |
| Actuated Green, G (s) | | 26.0 | 26.0 | | 26.0 | 63.0 | | 75.0 | 54.0 | 17.0 | 34.0 | 34.0 |
| Effective Green, g (s) | | 26.0 | 26.0 | | 26.0 | 63.0 | | 75.0 | 54.0 | 17.0 | 34.0 | 34.0 |
| Actuated g/C Ratio | | 0.24 | 0.24 | | 0.24 | 0.57 | | 0.68 | 0.49 | 0.15 | 0.31 | 0.31 |
| Clearance Time (s) | | 5.0 | 5.0 | | 5.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Grp Cap (vph) | | 440 | 374 | | 646 | 906 | | 957 | 777 | 273 | 489 | 489 |
| v/s Ratio Prot | | c0.11 | | | | 0.07 | | c0.17 | | 0.07 | 0.12 | 0.00 |
| v/s Ratio Perm | | | 0.04 | | 0.09 | 0.05 | | c0.17 | 0.01 | | | |
| v/c Ratio | | 0.46 | 0.18 | | 0.37 | 0.22 | | 0.49 | 0.02 | 0.47 | 0.37 | 0.01 |
| Uniform Delay, d1 | | 36.0 | 33.5 | | 35.2 | 11.5 | | 8.1 | 14.4 | 42.4 | 29.7 | 26.3 |
| Progression Factor | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | | 3.4 | 1.0 | | 1.6 | 0.6 | | 1.8 | 0.1 | 5.7 | 2.2 | 0.0 |
| Delay (s) | | 39.4 | 34.5 | | 36.8 | 12.0 | | 9.9 | 14.5 | 48.1 | 31.9 | 26.3 |
| Level of Service | | D | C | | D | B | | A | B | D | C | C |
| Approach Delay (s) | | 36.6 | | | 22.2 | | | 10.2 | | 38.2 | | |
| Approach LOS | | D | | | C | | | B | | D | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | 25.3 | | | HCM 2000 Level of Service | | | C | | | | |
| HCM 2000 Volume to Capacity ratio | | 0.50 | | | | | | | | | | |
| Actuated Cycle Length (s) | | 110.0 | | | Sum of lost time (s) | | | 13.0 | | | | |
| Intersection Capacity Utilization | | 61.5% | | | ICU Level of Service | | | B | | | | |
| Analysis Period (min) | | 15 | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

11: Reisterstown Rd & Anoka Ave









12/20/2021

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations |  | |  |  |  |  |
| Traffic Volume (veh/h) | 0 | 0 | 0 | 488 | 468 | 0 |
| Future Volume (Veh/h) | 0 | 0 | 0 | 488 | 468 | 0 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 530 | 509 | 0 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 1039 | 509 | 509 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1039 | 509 | 509 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 100 | 100 | 100 | | | |
| cM capacity (veh/h) | 255 | 564 | 1056 | | | |
| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 | SB 2 | |
| Volume Total | 0 | 0 | 530 | 509 | 0 | |
| Volume Left | 0 | 0 | 0 | 0 | 0 | |
| Volume Right | 0 | 0 | 0 | 0 | 0 | |
| cSH | 1700 | 1700 | 1700 | 1700 | 1700 | |
| Volume to Capacity | 0.00 | 0.00 | 0.31 | 0.30 | 0.00 | |
| Queue Length 95th (ft) | 0 | 0 | 0 | 0 | 0 | |
| Control Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| Lane LOS | A | | | | | |
| Approach Delay (s) | 0.0 | 0.0 | | 0.0 | | |
| Approach LOS | A | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.0 | | | |
| Intersection Capacity Utilization | | | 29.0% | ICU Level of Service | | A |
| Analysis Period (min) | | | 15 | | | |

HCM Signalized Intersection Capacity Analysis

1: Linden Ave & Druid Lake Park Dr

12/20/2021

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations |  | | |  | | |
| Traffic Volume (vph) | 691 | 15 | 29 | 925 | 0 | 0 |
| Future Volume (vph) | 691 | 15 | 29 | 925 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.0 | | | 4.0 | | |
| Lane Util. Factor | 1.00 | | | 0.95 | | |
| Frt | 1.00 | | | 1.00 | | |
| Flt Protected | 1.00 | | | 1.00 | | |
| Satd. Flow (prot) | 1857 | | | 3534 | | |
| Flt Permitted | 1.00 | | | 0.86 | | |
| Satd. Flow (perm) | 1857 | | | 3044 | | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 751 | 16 | 32 | 1005 | 0 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 767 | 0 | 0 | 1037 | 0 | 0 |
| Turn Type | NA | | Perm | NA | | |
| Protected Phases | 6 | | | 2 | | |
| Permitted Phases | | | 2 | | | |
| Actuated Green, G (s) | 70.0 | | | 71.0 | | |
| Effective Green, g (s) | 70.0 | | | 71.0 | | |
| Actuated g/C Ratio | 0.58 | | | 0.59 | | |
| Clearance Time (s) | 5.0 | | | 4.0 | | |
| Lane Grp Cap (vph) | 1083 | | | 1801 | | |
| v/s Ratio Prot | c0.41 | | | | | |
| v/s Ratio Perm | | | | 0.34 | | |
| v/c Ratio | 0.71 | | | 0.58 | | |
| Uniform Delay, d1 | 17.7 | | | 15.2 | | |
| Progression Factor | 0.72 | | | 1.00 | | |
| Incremental Delay, d2 | 2.1 | | | 1.3 | | |
| Delay (s) | 15.0 | | | 16.5 | | |
| Level of Service | B | | | B | | |
| Approach Delay (s) | 15.0 | | | 16.5 | 0.0 | |
| Approach LOS | B | | | B | A | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 15.9 | | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | | | 0.46 | | | |
| Actuated Cycle Length (s) | | | 120.0 | | Sum of lost time (s) | 11.5 |
| Intersection Capacity Utilization | | | 49.9% | | ICU Level of Service | A |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

2: Eutaw PI & Druid Lake Park Dr

12/20/2021



| Movement | EBL | EBR | NWL | NWR | SWL | SWR |
|------------------------|-------|------|-------|------|------|------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 666 | 109 | 47 | 40 | 29 | 896 |
| Future Volume (vph) | 666 | 109 | 47 | 40 | 29 | 896 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.4 | | 4.4 | 4.4 | 4.4 | 4.4 |
| Lane Util. Factor | 1.00 | | 1.00 | 1.00 | 1.00 | 0.95 |
| Frt | 0.98 | | 1.00 | 0.85 | 0.86 | 0.85 |
| Flt Protected | 0.96 | | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1752 | | 1770 | 1583 | 1596 | 1504 |
| Flt Permitted | 0.96 | | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1752 | | 1770 | 1583 | 1596 | 1504 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 724 | 118 | 51 | 43 | 32 | 974 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 35 | 208 | 217 |
| Lane Group Flow (vph) | 842 | 0 | 51 | 8 | 301 | 280 |
| Turn Type | Prot | | Prot | Perm | Prot | Perm |
| Protected Phases | 2! | | 8 | | 6! | |
| Permitted Phases | | | | 8 | | 6 |
| Actuated Green, G (s) | 66.6 | | 23.6 | 23.6 | 67.6 | 67.6 |
| Effective Green, g (s) | 66.6 | | 23.6 | 23.6 | 67.6 | 67.6 |
| Actuated g/C Ratio | 0.55 | | 0.20 | 0.20 | 0.56 | 0.56 |
| Clearance Time (s) | 5.4 | | 4.4 | 4.4 | 4.4 | 4.4 |
| Lane Grp Cap (vph) | 972 | | 348 | 311 | 899 | 847 |
| v/s Ratio Prot | c0.48 | | c0.03 | | 0.19 | |
| v/s Ratio Perm | | | | 0.01 | | 0.19 |
| v/c Ratio | 0.87 | | 0.15 | 0.03 | 0.33 | 0.33 |
| Uniform Delay, d1 | 22.9 | | 39.9 | 38.9 | 14.1 | 14.1 |
| Progression Factor | 1.00 | | 1.00 | 1.00 | 0.40 | 0.52 |
| Incremental Delay, d2 | 10.2 | | 0.9 | 0.2 | 0.8 | 0.9 |
| Delay (s) | 33.1 | | 40.8 | 39.1 | 6.5 | 8.2 |
| Level of Service | C | | D | D | A | A |
| Approach Delay (s) | 33.1 | | 40.0 | | 7.3 | |
| Approach LOS | C | | D | | A | |

Intersection Summary

| | | | |
|-----------------------------------|-------|---------------------------|------|
| HCM 2000 Control Delay | 20.1 | HCM 2000 Level of Service | C |
| HCM 2000 Volume to Capacity ratio | 0.58 | | |
| Actuated Cycle Length (s) | 120.0 | Sum of lost time (s) | 14.3 |
| Intersection Capacity Utilization | 81.3% | ICU Level of Service | D |
| Analysis Period (min) | 15 | | |




















! Phase conflict between lane groups.

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

4: McCulloh St/Swann Dr & Druid Lake Park Dr











12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  |  | | |  |  | |  | | |  |  |
| Traffic Volume (vph) | 8 | 754 | 125 | 30 | 860 | 53 | 144 | 5 | 28 | 6 | 13 | 5 |
| Future Volume (vph) | 8 | 754 | 125 | 30 | 860 | 53 | 144 | 5 | 28 | 6 | 13 | 5 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.6 | 5.6 | | | 5.6 | | | 5.6 | | | 5.6 | |
| Lane Util. Factor | 1.00 | 1.00 | | | 0.95 | | | 1.00 | | | 1.00 | |
| Frt | 1.00 | 0.98 | | | 0.99 | | | 0.98 | | | 0.97 | |
| Flt Protected | 0.95 | 1.00 | | | 1.00 | | | 0.96 | | | 0.99 | |
| Satd. Flow (prot) | 1770 | 1823 | | | 3504 | | | 1752 | | | 1790 | |
| Flt Permitted | 0.19 | 1.00 | | | 0.61 | | | 0.75 | | | 0.92 | |
| Satd. Flow (perm) | 359 | 1823 | | | 2136 | | | 1362 | | | 1676 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 9 | 820 | 136 | 33 | 935 | 58 | 157 | 5 | 30 | 7 | 14 | 5 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 5 | 0 | 0 | 4 | 0 |
| Lane Group Flow (vph) | 9 | 956 | 0 | 0 | 1023 | 0 | 0 | 187 | 0 | 0 | 22 | 0 |
| Turn Type | Perm | NA | | Perm | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 6 | | | 2 | | | 4 | | | 8 | |
| Permitted Phases | 6 | | | 2 | | | 4 | | | 8 | | |
| Actuated Green, G (s) | 79.4 | 79.4 | | | 79.4 | | | 34.4 | | | 34.4 | |
| Effective Green, g (s) | 79.4 | 79.4 | | | 79.4 | | | 34.4 | | | 34.4 | |
| Actuated g/C Ratio | 0.53 | 0.53 | | | 0.53 | | | 0.23 | | | 0.23 | |
| Clearance Time (s) | 5.6 | 5.6 | | | 5.6 | | | 5.6 | | | 5.6 | |
| Lane Grp Cap (vph) | 190 | 964 | | | 1130 | | | 312 | | | 384 | |
| v/s Ratio Prot | | c0.52 | | | | | | | | | | |
| v/s Ratio Perm | 0.03 | | | | 0.48 | | | c0.14 | | | 0.01 | |
| v/c Ratio | 0.05 | 0.99 | | | 0.91 | | | 0.60 | | | 0.06 | |
| Uniform Delay, d1 | 17.0 | 35.0 | | | 31.9 | | | 51.7 | | | 45.1 | |
| Progression Factor | 0.18 | 0.29 | | | 1.00 | | | 1.00 | | | 1.00 | |
| Incremental Delay, d2 | 0.2 | 16.1 | | | 11.9 | | | 8.3 | | | 0.3 | |
| Delay (s) | 3.3 | 26.2 | | | 43.8 | | | 60.0 | | | 45.4 | |
| Level of Service | A | C | | | D | | | E | | | D | |
| Approach Delay (s) | | 26.0 | | | 43.8 | | | 60.0 | | | 45.4 | |
| Approach LOS | | C | | | D | | | E | | | D | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 37.5 | | | | HCM 2000 Level of Service | | | D | | |
| HCM 2000 Volume to Capacity ratio | | | 0.74 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 150.0 | | | | Sum of lost time (s) | | | 15.2 | | |
| Intersection Capacity Utilization | | | 73.3% | | | | ICU Level of Service | | | D | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

5: N Fulton St & Druid Lake Park Dr/Auchentoroly Terr


















12/20/2021

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Movement | NBL | NBR | SET | SER | NWL | NWT |
| Lane Configurations |  | |  | |  |  |
| Traffic Volume (vph) | 0 | 170 | 717 | 0 | 254 | 755 |
| Future Volume (vph) | 0 | 170 | 717 | 0 | 254 | 755 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.0 | | 5.6 | | 4.5 | 5.6 |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | 0.95 |
| Frt | 0.86 | | 1.00 | | 1.00 | 1.00 |
| Flt Protected | 1.00 | | 1.00 | | 0.95 | 1.00 |
| Satd. Flow (prot) | 1611 | | 1863 | | 1770 | 3539 |
| Flt Permitted | 1.00 | | 1.00 | | 0.95 | 1.00 |
| Satd. Flow (perm) | 1611 | | 1863 | | 1770 | 3539 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 185 | 779 | 0 | 276 | 821 |
| RTOR Reduction (vph) | 154 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 31 | 0 | 779 | 0 | 276 | 821 |
| Turn Type | Prot | | NA | | Prot | NA |
| Protected Phases | 7 | | 2 | | 1 | 6 |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 25.0 | | 58.4 | | 31.5 | 114.4 |
| Effective Green, g (s) | 25.0 | | 58.4 | | 31.5 | 114.4 |
| Actuated g/C Ratio | 0.17 | | 0.39 | | 0.21 | 0.76 |
| Clearance Time (s) | 5.0 | | 5.6 | | 4.5 | 5.6 |
| Lane Grp Cap (vph) | 268 | | 725 | | 371 | 2699 |
| v/s Ratio Prot | c0.02 | | c0.42 | | c0.16 | c0.23 |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 0.12 | | 1.07 | | 0.74 | 0.30 |
| Uniform Delay, d1 | 53.1 | | 45.8 | | 55.5 | 5.5 |
| Progression Factor | 1.00 | | 1.00 | | 1.13 | 1.03 |
| Incremental Delay, d2 | 0.9 | | 55.2 | | 6.5 | 0.1 |
| Delay (s) | 54.0 | | 101.0 | | 69.3 | 5.8 |
| Level of Service | D | | F | | E | A |
| Approach Delay (s) | 54.0 | | 101.0 | | | 21.8 |
| Approach LOS | D | | F | | | C |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 54.6 | | HCM 2000 Level of Service | D |
| HCM 2000 Volume to Capacity ratio | | | 0.73 | | | |
| Actuated Cycle Length (s) | | | 150.0 | | Sum of lost time (s) | 20.7 |
| Intersection Capacity Utilization | | | 76.1% | | ICU Level of Service | D |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

7: Gwynns Falls Pkwy & Auchentoroly Terr

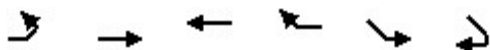
12/20/2021

| |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|
| Movement | SEL | SET | SER | NWR | NWR2 | NEL | NET | NER | SWT | SWR2 |
| Lane Configurations | |  |  |  | | |  |  |  |  |
| Traffic Volume (vph) | 13 | 427 | 294 | 753 | 2 | 33 | 41 | 300 | 11 | 8 |
| Future Volume (vph) | 13 | 427 | 294 | 753 | 2 | 33 | 41 | 300 | 11 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 6.0 | 5.0 | 6.0 | | | 6.0 | 5.0 | 6.0 | 6.0 |
| Lane Util. Factor | | 1.00 | 1.00 | 0.88 | | | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 1.00 | 0.85 | 0.85 | | | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | | 1.00 | 1.00 | 1.00 | | | 0.98 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 1860 | 1583 | 2787 | | | 1822 | 1583 | 1863 | 1583 |
| Flt Permitted | | 1.00 | 1.00 | 1.00 | | | 0.86 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 1860 | 1583 | 2787 | | | 1605 | 1583 | 1863 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 14 | 464 | 320 | 818 | 2 | 36 | 45 | 326 | 12 | 9 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 8 |
| Lane Group Flow (vph) | 0 | 478 | 320 | 812 | 0 | 0 | 81 | 326 | 12 | 1 |
| Turn Type | Perm | NA | custom | custom | | Perm | NA | custom | NA | Perm |
| Protected Phases | | 6 | 5 | 2 | | | 4 | 5 | 8 | |
| Permitted Phases | 6 | | 6 | 5 | | 4 | | 4 | | 8 |
| Actuated Green, G (s) | | 33.0 | 55.0 | 80.0 | | | 18.0 | 40.0 | 18.0 | 18.0 |
| Effective Green, g (s) | | 33.0 | 55.0 | 80.0 | | | 18.0 | 40.0 | 18.0 | 18.0 |
| Actuated g/C Ratio | | 0.30 | 0.50 | 0.73 | | | 0.16 | 0.36 | 0.16 | 0.16 |
| Clearance Time (s) | | 6.0 | 5.0 | 6.0 | | | 6.0 | 5.0 | 6.0 | 6.0 |
| Lane Grp Cap (vph) | | 558 | 863 | 2026 | | | 262 | 575 | 304 | 259 |
| v/s Ratio Prot | | | 0.07 | c0.29 | | | | c0.11 | 0.01 | |
| v/s Ratio Perm | | 0.26 | 0.13 | | | | 0.05 | 0.09 | | 0.00 |
| v/c Ratio | | 0.86 | 0.37 | 0.40 | | | 0.31 | 0.57 | 0.04 | 0.01 |
| Uniform Delay, d1 | | 36.3 | 16.9 | 5.8 | | | 40.5 | 28.1 | 38.7 | 38.5 |
| Progression Factor | | 0.61 | 0.52 | 1.00 | | | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | | 15.2 | 1.2 | 0.6 | | | 3.0 | 4.0 | 0.2 | 0.0 |
| Delay (s) | | 37.4 | 10.0 | 6.4 | | | 43.6 | 32.1 | 39.0 | 38.5 |
| Level of Service | | D | A | A | | | D | C | D | D |
| Approach Delay (s) | | 26.4 | | | | | 34.4 | | 38.8 | |
| Approach LOS | | C | | | | | C | | D | |
| Intersection Summary | | | | | | | | | | |
| HCM 2000 Control Delay | | | 20.1 | | | | HCM 2000 Level of Service | | C | |
| HCM 2000 Volume to Capacity ratio | | | 0.66 | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | | Sum of lost time (s) | | 23.0 | |
| Intersection Capacity Utilization | | | 75.3% | | | | ICU Level of Service | | D | |
| Analysis Period (min) | | | 15 | | | | | | | |
| c Critical Lane Group | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

8: Auchentoroly Terr & Auchentoroly Terr Uturn

12/20/2021






















| Movement | EBL | EBT | WBT | WBR | SEL | SER |
|-----------------------------------|------|-------|------|---------------------------|-------|------|
| Lane Configurations | | ↑ | | | ↔ | |
| Traffic Volume (vph) | 0 | 457 | 0 | 0 | 277 | 0 |
| Future Volume (vph) | 0 | 457 | 0 | 0 | 277 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 6.0 | | | 6.0 | |
| Lane Util. Factor | | 1.00 | | | 0.97 | |
| Frt | | 1.00 | | | 1.00 | |
| Flt Protected | | 1.00 | | | 0.95 | |
| Satd. Flow (prot) | | 1863 | | | 3433 | |
| Flt Permitted | | 1.00 | | | 0.95 | |
| Satd. Flow (perm) | | 1863 | | | 3433 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 497 | 0 | 0 | 301 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 282 | 0 |
| Lane Group Flow (vph) | 0 | 497 | 0 | 0 | 19 | 0 |
| Turn Type | | NA | | | Prot | |
| Protected Phases | | 2 | | | 3 | |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | | 91.0 | | | 7.0 | |
| Effective Green, g (s) | | 91.0 | | | 7.0 | |
| Actuated g/C Ratio | | 0.83 | | | 0.06 | |
| Clearance Time (s) | | 6.0 | | | 6.0 | |
| Vehicle Extension (s) | | 3.0 | | | 3.0 | |
| Lane Grp Cap (vph) | | 1541 | | | 218 | |
| v/s Ratio Prot | | c0.27 | | | c0.01 | |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | | 0.32 | | | 0.09 | |
| Uniform Delay, d1 | | 2.2 | | | 48.5 | |
| Progression Factor | | 4.13 | | | 1.00 | |
| Incremental Delay, d2 | | 0.5 | | | 0.2 | |
| Delay (s) | | 9.7 | | | 48.7 | |
| Level of Service | | A | | | D | |
| Approach Delay (s) | | 9.7 | 0.0 | | 48.7 | |
| Approach LOS | | A | A | | D | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | 24.4 | | HCM 2000 Level of Service | | C |
| HCM 2000 Volume to Capacity ratio | | 0.31 | | | | |
| Actuated Cycle Length (s) | | 110.0 | | Sum of lost time (s) | | 12.0 |
| Intersection Capacity Utilization | | 46.7% | | ICU Level of Service | | A |
| Analysis Period (min) | | 15 | | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

9: Liberty Heights Ave/Greenspring Ave & Auchentoroly Terr













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| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | |  | |  |  | | |  |  | |  |  |
| Traffic Volume (vph) | 0 | 254 | 5 | 137 | 322 | 58 | 9 | 41 | 162 | 41 | 81 | 1 |
| Future Volume (vph) | 0 | 254 | 5 | 137 | 322 | 58 | 9 | 41 | 162 | 41 | 81 | 1 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 6.0 | | 6.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Lane Util. Factor | | 1.00 | | 1.00 | 1.00 | | | 1.00 | 1.00 | | 0.95 | |
| Frt | | 1.00 | | 1.00 | 0.98 | | | 1.00 | 0.85 | | 1.00 | |
| Flt Protected | | 1.00 | | 0.95 | 1.00 | | | 0.99 | 1.00 | | 0.98 | |
| Satd. Flow (prot) | | 1858 | | 1770 | 1820 | | | 1846 | 1583 | | 3477 | |
| Flt Permitted | | 1.00 | | 0.95 | 1.00 | | | 0.94 | 1.00 | | 0.83 | |
| Satd. Flow (perm) | | 1858 | | 1770 | 1820 | | | 1744 | 1583 | | 2951 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 276 | 5 | 149 | 350 | 63 | 10 | 45 | 176 | 45 | 88 | 1 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 112 | 0 | 1 | 0 |
| Lane Group Flow (vph) | 0 | 281 | 0 | 149 | 407 | 0 | 0 | 55 | 64 | 0 | 133 | 0 |
| Turn Type | | NA | | Prot | NA | | Perm | NA | pm+ov | custom | NA | |
| Protected Phases | | 6 | | 5 | 2 | | | 4 | 5 | | | |
| Permitted Phases | | | | | | | 4 | | 4 | 8 | 8 | |
| Actuated Green, G (s) | | 27.0 | | 21.0 | 79.0 | | | 19.0 | 40.0 | | 19.0 | |
| Effective Green, g (s) | | 27.0 | | 21.0 | 79.0 | | | 19.0 | 40.0 | | 19.0 | |
| Actuated g/C Ratio | | 0.25 | | 0.19 | 0.72 | | | 0.17 | 0.36 | | 0.17 | |
| Clearance Time (s) | | 6.0 | | 6.0 | 6.0 | | | 6.0 | 6.0 | | 6.0 | |
| Lane Grp Cap (vph) | | 456 | | 337 | 1307 | | | 301 | 661 | | 509 | |
| v/s Ratio Prot | | c0.15 | | c0.08 | c0.22 | | | | 0.02 | | | |
| v/s Ratio Perm | | | | | | | | 0.03 | 0.02 | | c0.05 | |
| v/c Ratio | | 0.62 | | 0.44 | 0.31 | | | 0.18 | 0.10 | | 0.26 | |
| Uniform Delay, d1 | | 36.9 | | 39.3 | 5.6 | | | 38.9 | 23.1 | | 39.4 | |
| Progression Factor | | 0.94 | | 0.91 | 0.95 | | | 1.84 | 0.64 | | 1.00 | |
| Incremental Delay, d2 | | 6.0 | | 3.9 | 0.6 | | | 1.3 | 0.3 | | 1.2 | |
| Delay (s) | | 40.6 | | 39.7 | 5.9 | | | 72.9 | 15.0 | | 40.7 | |
| Level of Service | | D | | D | A | | | E | B | | D | |
| Approach Delay (s) | | 40.6 | | | 14.9 | | | 28.8 | | | 40.7 | |
| Approach LOS | | D | | | B | | | C | | | D | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 26.4 | | | HCM 2000 Level of Service | | | C | | | |
| HCM 2000 Volume to Capacity ratio | | | 0.44 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | Sum of lost time (s) | | | 24.0 | | | |
| Intersection Capacity Utilization | | | 45.6% | | | ICU Level of Service | | | A | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

10: Reisterstown Rd & Liberty Heights Ave











12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|---|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SBL2 | SBL | SBR | NWL | NWR | NWR2 |
| Lane Configurations | | ↑↑ | ↑ | ↑ | ↑↑ | | | ↑ | ↑ | ↑ | ↑ | ↑ |
| Traffic Volume (vph) | 0 | 175 | 257 | 37 | 184 | 2 | 5 | 204 | 15 | 118 | 169 | 12 |
| Future Volume (vph) | 0 | 175 | 257 | 37 | 184 | 2 | 5 | 204 | 15 | 118 | 169 | 12 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 5.0 | 4.0 | 5.0 | 5.0 | | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | | 0.95 | 1.00 | 1.00 | 0.95 | | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 1.00 | 0.85 | 1.00 | 1.00 | | | 1.00 | 0.85 | 1.00 | 0.85 | 0.85 |
| Flt Protected | | 1.00 | 1.00 | 0.95 | 1.00 | | | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 3539 | 1583 | 1770 | 3534 | | | 1770 | 1583 | 1770 | 1583 | 1583 |
| Flt Permitted | | 1.00 | 1.00 | 0.59 | 1.00 | | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 3539 | 1583 | 1099 | 3534 | | | 1855 | 1583 | 1863 | 1583 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 190 | 279 | 40 | 200 | 2 | 5 | 222 | 16 | 128 | 184 | 13 |
| RTOR Reduction (vph) | 0 | 0 | 231 | 0 | 1 | 0 | 0 | 0 | 5 | 0 | 0 | 2 |
| Lane Group Flow (vph) | 0 | 190 | 48 | 40 | 201 | 0 | 0 | 227 | 11 | 128 | 184 | 11 |
| Turn Type | | NA | pm+ov | Perm | NA | | Perm | Prot | Perm | pm+pt | Prot | Perm |
| Protected Phases | | 4 | 1 | | 8 | | | 6 | | 1 | 2 | |
| Permitted Phases | | | 4 | 8 | | | 6 | | 6 | 2 | | 2 |
| Actuated Green, G (s) | | 11.7 | 18.9 | 11.7 | 11.7 | | | 78.1 | 78.1 | 89.3 | 89.3 | 89.3 |
| Effective Green, g (s) | | 11.7 | 18.9 | 11.7 | 11.7 | | | 78.1 | 78.1 | 89.3 | 89.3 | 89.3 |
| Actuated g/C Ratio | | 0.11 | 0.17 | 0.11 | 0.11 | | | 0.71 | 0.71 | 0.81 | 0.81 | 0.81 |
| Clearance Time (s) | | 5.0 | 4.0 | 5.0 | 5.0 | | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | | 3.0 | 3.0 | 3.0 | 3.0 | | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | | 376 | 271 | 116 | 375 | | | 1317 | 1123 | 1512 | 1285 | 1285 |
| v/s Ratio Prot | | 0.05 | c0.01 | | c0.06 | | | | | 0.01 | 0.12 | |
| v/s Ratio Perm | | | 0.02 | 0.04 | | | | c0.12 | 0.01 | 0.06 | | 0.01 |
| v/c Ratio | | 0.51 | 0.18 | 0.34 | 0.54 | | | 0.17 | 0.01 | 0.08 | 0.14 | 0.01 |
| Uniform Delay, d1 | | 46.4 | 38.9 | 45.6 | 46.6 | | | 5.3 | 4.7 | 2.2 | 2.2 | 2.0 |
| Progression Factor | | 1.00 | 1.00 | 1.13 | 1.14 | | | 0.96 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | | 1.1 | 0.3 | 1.7 | 1.4 | | | 0.3 | 0.0 | 0.0 | 0.2 | 0.0 |
| Delay (s) | | 47.5 | 39.2 | 53.0 | 54.6 | | | 5.3 | 4.7 | 2.3 | 2.4 | 2.0 |
| Level of Service | | D | D | D | D | | | A | A | A | A | A |
| Approach Delay (s) | | 42.6 | | | 54.3 | | | 5.3 | | 2.4 | | |
| Approach LOS | | D | | | D | | | A | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 27.5 | | | HCM 2000 Level of Service | | | | C | | |
| HCM 2000 Volume to Capacity ratio | | | 0.22 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | Sum of lost time (s) | | | | 13.0 | | |
| Intersection Capacity Utilization | | | 43.1% | | | ICU Level of Service | | | | A | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

11: Reisterstown Rd & Anoka Ave

12/20/2021

| | | | | | | |
|-----------------------------------|---|---|---|---|---|---|
| |  |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations |  | |  |  |  | |
| Traffic Volume (veh/h) | 0 | 0 | 0 | 488 | 468 | 0 |
| Future Volume (Veh/h) | 0 | 0 | 0 | 488 | 468 | 0 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 530 | 509 | 0 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage (veh) | | | | | | |
| Upstream signal (ft) | | | | 382 | | |
| pX, platoon unblocked | 0.99 | | | | | |
| vC, conflicting volume | 1039 | 509 | 509 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1032 | 509 | 509 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 100 | 100 | 100 | | | |
| cM capacity (veh/h) | 254 | 564 | 1056 | | | |
| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 | | |
| Volume Total | 0 | 0 | 530 | 509 | | |
| Volume Left | 0 | 0 | 0 | 0 | | |
| Volume Right | 0 | 0 | 0 | 0 | | |
| cSH | 1700 | 1700 | 1700 | 1700 | | |
| Volume to Capacity | 0.00 | 0.00 | 0.31 | 0.30 | | |
| Queue Length 95th (ft) | 0 | 0 | 0 | 0 | | |
| Control Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | | |
| Lane LOS | A | | | | | |
| Approach Delay (s) | 0.0 | 0.0 | | 0.0 | | |
| Approach LOS | A | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.0 | | | |
| Intersection Capacity Utilization | | | 29.0% | ICU Level of Service | | A |
| Analysis Period (min) | | | 15 | | | |

HCM Signalized Intersection Capacity Analysis

1: Linden Ave & Druid Lake Park Dr

12/20/2021

| | → | ↘ | ↙ | ← | ↖ | ↗ |
|-----------------------------------|-------|------|-------|------|---------------------------|------|
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | ↰ | | | ↱ | | |
| Traffic Volume (vph) | 691 | 15 | 44 | 910 | 0 | 0 |
| Future Volume (vph) | 691 | 15 | 44 | 910 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.0 | | | 4.0 | | |
| Lane Util. Factor | 1.00 | | | 0.95 | | |
| Frt | 1.00 | | | 1.00 | | |
| Flt Protected | 1.00 | | | 1.00 | | |
| Satd. Flow (prot) | 1857 | | | 3531 | | |
| Flt Permitted | 1.00 | | | 0.79 | | |
| Satd. Flow (perm) | 1857 | | | 2780 | | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 751 | 16 | 48 | 989 | 0 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 767 | 0 | 0 | 1037 | 0 | 0 |
| Turn Type | NA | | Perm | NA | | |
| Protected Phases | 6 | | | 2 | | |
| Permitted Phases | | | 2 | | | |
| Actuated Green, G (s) | 70.0 | | | 71.0 | | |
| Effective Green, g (s) | 70.0 | | | 71.0 | | |
| Actuated g/C Ratio | 0.58 | | | 0.59 | | |
| Clearance Time (s) | 5.0 | | | 4.0 | | |
| Lane Grp Cap (vph) | 1083 | | | 1644 | | |
| v/s Ratio Prot | c0.41 | | | | | |
| v/s Ratio Perm | | | | 0.37 | | |
| v/c Ratio | 0.71 | | | 0.63 | | |
| Uniform Delay, d1 | 17.7 | | | 16.0 | | |
| Progression Factor | 1.00 | | | 1.00 | | |
| Incremental Delay, d2 | 3.9 | | | 1.8 | | |
| Delay (s) | 21.7 | | | 17.8 | | |
| Level of Service | C | | | B | | |
| Approach Delay (s) | 21.7 | | | 17.8 | 0.0 | |
| Approach LOS | C | | | B | A | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 19.5 | | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | | | 0.46 | | | |
| Actuated Cycle Length (s) | | | 120.0 | | Sum of lost time (s) | 11.5 |
| Intersection Capacity Utilization | | | 61.0% | | ICU Level of Service | B |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

2: Eutaw PI & Druid Lake Park Dr

12/20/2021



















| Movement | EBL | EBR | NWL | NWR | SWL | SWR |
|-----------------------------------|-------|------|-------|------|---------------------------|------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 652 | 117 | 119 | 54 | 44 | 866 |
| Future Volume (vph) | 652 | 117 | 119 | 54 | 44 | 866 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.6 | | 5.6 | 5.6 | 5.6 | 5.6 |
| Lane Util. Factor | 1.00 | | 1.00 | 1.00 | 1.00 | 0.95 |
| Frt | 0.98 | | 1.00 | 0.85 | 0.87 | 0.85 |
| Flt Protected | 0.96 | | 0.95 | 1.00 | 0.99 | 1.00 |
| Satd. Flow (prot) | 1750 | | 1770 | 1583 | 1606 | 1504 |
| Flt Permitted | 0.96 | | 0.95 | 1.00 | 0.57 | 1.00 |
| Satd. Flow (perm) | 1750 | | 1770 | 1583 | 917 | 1504 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 709 | 127 | 129 | 59 | 48 | 941 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 49 | 151 | 236 |
| Lane Group Flow (vph) | 836 | 0 | 129 | 10 | 264 | 338 |
| Turn Type | Prot | | Prot | Prot | Perm | Prot |
| Protected Phases | 2 | | 8 | 8 | | 6 |
| Permitted Phases | | | | | 6 | |
| Actuated Green, G (s) | 88.4 | | 25.4 | 25.4 | 88.4 | 88.4 |
| Effective Green, g (s) | 88.4 | | 25.4 | 25.4 | 88.4 | 88.4 |
| Actuated g/C Ratio | 0.59 | | 0.17 | 0.17 | 0.59 | 0.59 |
| Clearance Time (s) | 5.6 | | 5.6 | 5.6 | 5.6 | 5.6 |
| Lane Grp Cap (vph) | 1031 | | 299 | 268 | 540 | 886 |
| v/s Ratio Prot | c0.48 | | c0.07 | 0.01 | | 0.22 |
| v/s Ratio Perm | | | | | 0.29 | |
| v/c Ratio | 0.81 | | 0.43 | 0.04 | 0.49 | 0.38 |
| Uniform Delay, d1 | 24.2 | | 55.8 | 52.1 | 17.8 | 16.3 |
| Progression Factor | 0.10 | | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 3.0 | | 4.5 | 0.3 | 3.2 | 1.2 |
| Delay (s) | 5.5 | | 60.3 | 52.3 | 20.9 | 17.6 |
| Level of Service | A | | E | D | C | B |
| Approach Delay (s) | 5.5 | | 57.8 | | 19.0 | |
| Approach LOS | A | | E | | B | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 17.0 | | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | | | 0.61 | | | |
| Actuated Cycle Length (s) | | | 150.0 | | Sum of lost time (s) | 15.2 |
| Intersection Capacity Utilization | | | 90.0% | | ICU Level of Service | E |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis

3: Madison Ave/Swann Dr & Druid Lake Park Dr











12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SEL | SET | SER | NWL | NWT | NWR |
| Lane Configurations | |  | | |  | | |  | | |  | |
| Traffic Volume (vph) | 8 | 749 | 130 | 127 | 805 | 53 | 6 | 13 | 5 | 72 | 0 | 14 |
| Future Volume (vph) | 8 | 749 | 130 | 127 | 805 | 53 | 6 | 13 | 5 | 72 | 0 | 14 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 4.6 | | | 4.6 | | | 5.6 | | | 5.6 | |
| Lane Util. Factor | | 1.00 | | | 0.95 | | | 1.00 | | | 1.00 | |
| Frt | | 0.98 | | | 0.99 | | | 0.97 | | | 0.98 | |
| Flt Protected | | 1.00 | | | 0.99 | | | 0.99 | | | 0.96 | |
| Satd. Flow (prot) | | 1825 | | | 3488 | | | 1790 | | | 1749 | |
| Flt Permitted | | 0.99 | | | 0.60 | | | 0.93 | | | 0.74 | |
| Satd. Flow (perm) | | 1803 | | | 2092 | | | 1696 | | | 1353 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 9 | 814 | 141 | 138 | 875 | 58 | 7 | 14 | 5 | 78 | 0 | 15 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 4 | 0 | 0 | 36 | 0 |
| Lane Group Flow (vph) | 0 | 964 | 0 | 0 | 1068 | 0 | 0 | 22 | 0 | 0 | 57 | 0 |
| Turn Type | Perm | NA | | Perm | NA | | Perm | NA | | Perm | NA | |
| Protected Phases | | 6 | | | 2 | | | 4 | | | 8 | |
| Permitted Phases | 6 | | | 2 | | | 4 | | | 8 | | |
| Actuated Green, G (s) | | 87.4 | | | 87.4 | | | 27.4 | | | 27.4 | |
| Effective Green, g (s) | | 87.4 | | | 87.4 | | | 27.4 | | | 27.4 | |
| Actuated g/C Ratio | | 0.58 | | | 0.58 | | | 0.18 | | | 0.18 | |
| Clearance Time (s) | | 4.6 | | | 4.6 | | | 5.6 | | | 5.6 | |
| Lane Grp Cap (vph) | | 1050 | | | 1218 | | | 309 | | | 247 | |
| v/s Ratio Prot | | | | | | | | | | | | |
| v/s Ratio Perm | | c0.53 | | | 0.51 | | | 0.01 | | | c0.04 | |
| v/c Ratio | | 0.92 | | | 0.88 | | | 0.07 | | | 0.23 | |
| Uniform Delay, d1 | | 28.1 | | | 26.7 | | | 50.8 | | | 52.3 | |
| Progression Factor | | 1.00 | | | 1.14 | | | 1.00 | | | 1.00 | |
| Incremental Delay, d2 | | 13.9 | | | 7.9 | | | 0.4 | | | 2.2 | |
| Delay (s) | | 42.0 | | | 38.2 | | | 51.2 | | | 54.5 | |
| Level of Service | | D | | | D | | | D | | | D | |
| Approach Delay (s) | | 42.0 | | | 38.2 | | | 51.2 | | | 54.5 | |
| Approach LOS | | D | | | D | | | D | | | D | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 40.8 | | | HCM 2000 Level of Service | | | | D | | |
| HCM 2000 Volume to Capacity ratio | | | 0.64 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 150.0 | | | Sum of lost time (s) | | | 14.7 | | | |
| Intersection Capacity Utilization | | | 99.2% | | | ICU Level of Service | | | F | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis

5: N Fulton St & Druid Lake Park Dr/Auchentoroly Terr




















12/20/2021

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Movement | NBL | NBR | SET | SER | NWL | NWT |
| Lane Configurations | |  |  |  | |  |
| Traffic Volume (veh/h) | 0 | 170 | 717 | 0 | 0 | 882 |
| Future Volume (Veh/h) | 0 | 170 | 717 | 0 | 0 | 882 |
| Sign Control | Stop | | Free | | | Free |
| Grade | 0% | | 0% | | | 0% |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 185 | 779 | 0 | 0 | 959 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | None | | | None | |
| Median storage veh | | | | | | |
| Upstream signal (ft) | | | | | | 812 |
| pX, platoon unblocked | 0.84 | | | | | |
| vC, conflicting volume | 1258 | 779 | | | 779 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 933 | 779 | | | 779 | |
| tC, single (s) | 6.8 | 6.9 | | | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | | | 2.2 | |
| p0 queue free % | 100 | 45 | | | 100 | |
| cM capacity (veh/h) | 223 | 339 | | | 834 | |
| Direction, Lane # | NB 1 | SE 1 | SE 2 | NW 1 | NW 2 | |
| Volume Total | 185 | 779 | 0 | 480 | 480 | |
| Volume Left | 0 | 0 | 0 | 0 | 0 | |
| Volume Right | 185 | 0 | 0 | 0 | 0 | |
| cSH | 339 | 1700 | 1700 | 1700 | 1700 | |
| Volume to Capacity | 0.55 | 0.46 | 0.00 | 0.28 | 0.28 | |
| Queue Length 95th (ft) | 78 | 0 | 0 | 0 | 0 | |
| Control Delay (s) | 27.7 | 0.0 | 0.0 | 0.0 | 0.0 | |
| Lane LOS | D | | | | | |
| Approach Delay (s) | 27.7 | 0.0 | | 0.0 | | |
| Approach LOS | D | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | 2.7 | | | | |
| Intersection Capacity Utilization | | 54.9% | | ICU Level of Service | | A |
| Analysis Period (min) | | 15 | | | | |

HCM Signalized Intersection Capacity Analysis

7: Gwynns Falls Pkwy & Auchentoroly Terr


12/20/2021

| |  |  |  |  |  |  |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|--|---|---|---|---|---|
| Movement | SEL | SET | SER | SER2 | NWL | NWT | NWR | NEL | NET | NER | SWT | SWR |
| Lane Configurations | |  | | |  |  | | |  |  |  |  |
| Traffic Volume (vph) | 13 | 417 | 10 | 17 | 404 | 476 | 2 | 33 | 41 | 300 | 11 | 8 |
| Future Volume (vph) | 13 | 417 | 10 | 17 | 404 | 476 | 2 | 33 | 41 | 300 | 11 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 6.0 | | | 5.0 | 6.0 | | | 6.0 | 5.0 | 6.0 | |
| Lane Util. Factor | | 0.95 | | | 1.00 | 0.95 | | | 1.00 | 1.00 | 1.00 | |
| Frt | | 0.99 | | | 1.00 | 1.00 | | | 1.00 | 0.85 | 0.94 | |
| Flt Protected | | 1.00 | | | 0.95 | 1.00 | | | 0.98 | 1.00 | 1.00 | |
| Satd. Flow (prot) | | 3503 | | | 1770 | 3537 | | | 1822 | 1583 | 1755 | |
| Flt Permitted | | 0.93 | | | 0.34 | 1.00 | | | 0.86 | 1.00 | 1.00 | |
| Satd. Flow (perm) | | 3258 | | | 625 | 3537 | | | 1597 | 1583 | 1755 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 14 | 453 | 11 | 18 | 439 | 517 | 2 | 36 | 45 | 326 | 12 | 9 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 |
| Lane Group Flow (vph) | 0 | 496 | 0 | 0 | 439 | 519 | 0 | 0 | 81 | 326 | 13 | 0 |
| Turn Type | Perm | NA | | | pm+pt | NA | | | Perm | NA | pm+ov | NA |
| Protected Phases | | 6 | | | 5 | 2 | | | 4 | 5 | 8 | |
| Permitted Phases | 6 | | | | 2 | | | 4 | | 4 | | |
| Actuated Green, G (s) | | 23.0 | | | 80.0 | 80.0 | | | 18.0 | 50.0 | 18.0 | |
| Effective Green, g (s) | | 23.0 | | | 80.0 | 80.0 | | | 18.0 | 50.0 | 18.0 | |
| Actuated g/C Ratio | | 0.21 | | | 0.73 | 0.73 | | | 0.16 | 0.45 | 0.16 | |
| Clearance Time (s) | | 6.0 | | | 5.0 | 6.0 | | | 6.0 | 5.0 | 6.0 | |
| Lane Grp Cap (vph) | | 681 | | | 787 | 2572 | | | 261 | 719 | 287 | |
| v/s Ratio Prot | | | | | c0.16 | 0.15 | | | | c0.13 | 0.01 | |
| v/s Ratio Perm | | c0.15 | | | c0.24 | | | | 0.05 | 0.07 | | |
| v/c Ratio | | 0.73 | | | 0.56 | 0.20 | | | 0.31 | 0.45 | 0.05 | |
| Uniform Delay, d1 | | 40.6 | | | 7.2 | 4.8 | | | 40.5 | 20.6 | 38.8 | |
| Progression Factor | | 1.00 | | | 1.00 | 1.00 | | | 1.00 | 1.00 | 1.00 | |
| Incremental Delay, d2 | | 6.7 | | | 2.8 | 0.2 | | | 3.1 | 2.1 | 0.3 | |
| Delay (s) | | 47.3 | | | 10.0 | 5.0 | | | 43.6 | 22.7 | 39.1 | |
| Level of Service | | D | | | B | A | | | D | C | D | |
| Approach Delay (s) | | 47.3 | | | | 7.3 | | | 26.8 | | 39.1 | |
| Approach LOS | | D | | | | A | | | C | | D | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | 22.4 | | | HCM 2000 Level of Service | | | | C | | | |
| HCM 2000 Volume to Capacity ratio | | 0.60 | | | | | | | | | | |
| Actuated Cycle Length (s) | | 110.0 | | | Sum of lost time (s) | | | | 23.0 | | | |
| Intersection Capacity Utilization | | 60.8% | | | ICU Level of Service | | | | B | | | |
| Analysis Period (min) | | 15 | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis

10: Reisterstown Rd & Liberty Heights Ave

12/20/2021











| |  | | | | | | | | | | | |
|-----------------------------------|--|------|-------|------|-------|---------------------------|------|-------|------|-------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | SBL2 | SBL | SBR | NWL | NWR | NWR2 |
| Lane Configurations | | ↑↑ | ↑ | ↑ | ↑↑ | | | ↑ | ↑ | ↑ | ↑ | ↑ |
| Traffic Volume (vph) | 0 | 175 | 257 | 37 | 184 | 2 | 5 | 204 | 15 | 118 | 169 | 12 |
| Future Volume (vph) | 0 | 175 | 257 | 37 | 184 | 2 | 5 | 204 | 15 | 118 | 169 | 12 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | | 5.0 | 4.0 | 5.0 | 5.0 | | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | | 0.95 | 1.00 | 1.00 | 0.95 | | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 1.00 | 0.85 | 1.00 | 1.00 | | | 1.00 | 0.85 | 1.00 | 0.85 | 0.85 |
| Flt Protected | | 1.00 | 1.00 | 0.95 | 1.00 | | | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | | 3539 | 1583 | 1770 | 3534 | | | 1770 | 1583 | 1770 | 1583 | 1583 |
| Flt Permitted | | 1.00 | 1.00 | 0.59 | 1.00 | | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | | 3539 | 1583 | 1099 | 3534 | | | 1855 | 1583 | 1863 | 1583 | 1583 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 190 | 279 | 40 | 200 | 2 | 5 | 222 | 16 | 128 | 184 | 13 |
| RTOR Reduction (vph) | 0 | 0 | 231 | 0 | 1 | 0 | 0 | 0 | 5 | 0 | 0 | 2 |
| Lane Group Flow (vph) | 0 | 190 | 48 | 40 | 201 | 0 | 0 | 227 | 11 | 128 | 184 | 11 |
| Turn Type | | NA | pm+ov | Perm | NA | | Perm | Prot | Perm | pm+pt | Prot | Perm |
| Protected Phases | | 4 | 1 | | 8 | | | 6 | | 1 | 2 | |
| Permitted Phases | | | 4 | 8 | | | 6 | | 6 | 2 | | 2 |
| Actuated Green, G (s) | | 11.7 | 18.9 | 11.7 | 11.7 | | | 78.1 | 78.1 | 89.3 | 89.3 | 89.3 |
| Effective Green, g (s) | | 11.7 | 18.9 | 11.7 | 11.7 | | | 78.1 | 78.1 | 89.3 | 89.3 | 89.3 |
| Actuated g/C Ratio | | 0.11 | 0.17 | 0.11 | 0.11 | | | 0.71 | 0.71 | 0.81 | 0.81 | 0.81 |
| Clearance Time (s) | | 5.0 | 4.0 | 5.0 | 5.0 | | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | | 3.0 | 3.0 | 3.0 | 3.0 | | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | | 376 | 271 | 116 | 375 | | | 1317 | 1123 | 1512 | 1285 | 1285 |
| v/s Ratio Prot | | 0.05 | c0.01 | | c0.06 | | | | | 0.01 | 0.12 | |
| v/s Ratio Perm | | | 0.02 | 0.04 | | | | c0.12 | 0.01 | 0.06 | | 0.01 |
| v/c Ratio | | 0.51 | 0.18 | 0.34 | 0.54 | | | 0.17 | 0.01 | 0.08 | 0.14 | 0.01 |
| Uniform Delay, d1 | | 46.4 | 38.9 | 45.6 | 46.6 | | | 5.3 | 4.7 | 2.2 | 2.2 | 2.0 |
| Progression Factor | | 1.00 | 1.00 | 1.00 | 1.00 | | | 0.99 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | | 1.1 | 0.3 | 1.8 | 1.5 | | | 0.3 | 0.0 | 0.0 | 0.2 | 0.0 |
| Delay (s) | | 47.5 | 39.2 | 47.4 | 48.1 | | | 5.5 | 4.7 | 2.3 | 2.4 | 2.0 |
| Level of Service | | D | D | D | D | | | A | A | A | A | A |
| Approach Delay (s) | | 42.6 | | | 47.9 | | | 5.5 | | 2.4 | | |
| Approach LOS | | D | | | D | | | A | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 2000 Control Delay | | | 26.3 | | | HCM 2000 Level of Service | | | | C | | |
| HCM 2000 Volume to Capacity ratio | | | 0.22 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | | Sum of lost time (s) | | | 13.0 | | | |
| Intersection Capacity Utilization | | | 43.1% | | | ICU Level of Service | | | A | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

c Critical Lane Group

HCM Unsignalized Intersection Capacity Analysis

11: Reisterstown Rd & Anoka Ave

12/20/2021










| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations |  | |  |  |  | |
| Traffic Volume (veh/h) | 0 | 0 | 0 | 488 | 468 | 0 |
| Future Volume (Veh/h) | 0 | 0 | 0 | 488 | 468 | 0 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 530 | 509 | 0 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh | | | | | | |
| Upstream signal (ft) | | | | 382 | | |
| pX, platoon unblocked | 0.98 | | | | | |
| vC, conflicting volume | 1039 | 254 | 509 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 1031 | 254 | 509 | | | |
| tC, single (s) | 6.8 | 6.9 | 4.1 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | |
| p0 queue free % | 100 | 100 | 100 | | | |
| cM capacity (veh/h) | 225 | 745 | 1052 | | | |
| Direction, Lane # | EB 1 | NB 1 | NB 2 | SB 1 | SB 2 | |
| Volume Total | 0 | 0 | 530 | 339 | 170 | |
| Volume Left | 0 | 0 | 0 | 0 | 0 | |
| Volume Right | 0 | 0 | 0 | 0 | 0 | |
| cSH | 1700 | 1700 | 1700 | 1700 | 1700 | |
| Volume to Capacity | 0.00 | 0.00 | 0.31 | 0.20 | 0.10 | |
| Queue Length 95th (ft) | 0 | 0 | 0 | 0 | 0 | |
| Control Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| Lane LOS | A | | | | | |
| Approach Delay (s) | 0.0 | 0.0 | | 0.0 | | |
| Approach LOS | A | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 0.0 | | | |
| Intersection Capacity Utilization | | | 29.0% | ICU Level of Service | | A |
| Analysis Period (min) | | | 15 | | | |

DPLD Concepts without Volume Reductions (PM)

HCM Signalized Intersection Capacity Analysis

1: Linden Ave & Druid Lake Park Dr

12/20/2021

| |  |  |  |  |  |  |
|-----------------------------------|---|---|---|---|---|---|
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations |  | |  |  | | |
| Traffic Volume (vph) | 968 | 30 | 67 | 1415 | 0 | 0 |
| Future Volume (vph) | 968 | 30 | 67 | 1415 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 5.0 | | 4.5 | 5.0 | | |
| Lane Util. Factor | 1.00 | | 1.00 | 1.00 | | |
| Frt | 1.00 | | 1.00 | 1.00 | | |
| Flt Protected | 1.00 | | 0.95 | 1.00 | | |
| Satd. Flow (prot) | 1855 | | 1770 | 1863 | | |
| Flt Permitted | 1.00 | | 0.95 | 1.00 | | |
| Satd. Flow (perm) | 1855 | | 1770 | 1863 | | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 1052 | 33 | 73 | 1538 | 0 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 1085 | 0 | 73 | 1538 | 0 | 0 |
| Turn Type | NA | | Prot | NA | | |
| Protected Phases | 6 | | 5 | 2 | | |
| Permitted Phases | | | | | | |
| Actuated Green, G (s) | 81.0 | | 9.5 | 120.0 | | |
| Effective Green, g (s) | 81.0 | | 9.5 | 120.0 | | |
| Actuated g/C Ratio | 0.54 | | 0.06 | 0.80 | | |
| Clearance Time (s) | 5.0 | | 4.5 | 5.0 | | |
| Lane Grp Cap (vph) | 1001 | | 112 | 1490 | | |
| v/s Ratio Prot | 0.58 | | 0.04 | c0.83 | | |
| v/s Ratio Perm | | | | | | |
| v/c Ratio | 1.08 | | 0.65 | 1.03 | | |
| Uniform Delay, d1 | 34.5 | | 68.6 | 15.0 | | |
| Progression Factor | 0.24 | | 1.00 | 1.00 | | |
| Incremental Delay, d2 | 39.7 | | 25.8 | 32.0 | | |
| Delay (s) | 48.1 | | 94.5 | 47.0 | | |
| Level of Service | D | | F | D | | |
| Approach Delay (s) | 48.1 | | | 49.2 | 0.0 | |
| Approach LOS | D | | | D | A | |
| Intersection Summary | | | | | | |
| HCM 2000 Control Delay | | | 48.7 | HCM 2000 Level of Service | | D |
| HCM 2000 Volume to Capacity ratio | | | 0.93 | | | |
| Actuated Cycle Length (s) | | | 150.0 | Sum of lost time (s) | | 17.5 |
| Intersection Capacity Utilization | | | 78.6% | ICU Level of Service | | D |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |